

SHEET INDEX:
S1 - GENERAL NOTES
S2 - FOUNDATION PLANS
S3 - FOUNDATION SECTION, ANCHOR DETAILS
GENERAL STRUCTURAL NOTES

1. THESE NOTES ARE GENERAL IN NATURE AND ARE INTENDED TO SET MINIMUM STANDARDS FOR CONSTRUCTION. WHERE CONFLICTS BETWEEN THE DRAWINGS EXIST, THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL EOR. THE CONTRACTOR SHALL BE COMPLETELY FAMILIAR WITH THE FINAL CONTRACT DOCUMENTS AND HAVE A COPY OF THEM ON SITE AT ALL TIMES. ELECTRONIC EQUIVALENTS ARE ACCEPTABLE WHERE PERMITTED BY THE GOVERNING JURISDICTION.
2. THESE DRAWINGS HAVE BEEN PREPARED SOLELY FOR USE IN THE CONSTRUCTION OF THE OMIC BUILDING 1 MACHINE FOUNDATION PROJECT LOCATED IN SCAPPOOSE, OREGON. POSSESSION OF THESE DRAWINGS DOES NOT CONSTITUTE A LICENSE TO CONSTRUCT OR FABRICATE THE WHOLE, OR PARTS OF THIS PROJECT IN OTHER LOCATIONS.
3. SCALES NOTED ON DRAWINGS ARE FOR GENERAL INFORMATION ONLY. DO NOT SCALE DRAWINGS.
4. ALL WORK SHALL BE IN STRICT CONFORMANCE WITH THE 2021 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED BY THE 2022 OREGON STRUCTURAL SPECIALTY CODE (OSSC),
5. ALL STANDARDS AND SPECIFICATIONS REFERENCED WITHIN THESE DRAWINGS REFER TO THE MOST CURRENT VERSION REFERENCED IN THE SPECIFIED BUILDING CODE OR THE MOST CURRENT VERSION AVAILABLE AT THE PUBLISH DATE OF THESE DRAWINGS.
6. FOR ANY PORTION OF THE CONSTRUCTION WHICH THE CONTRACTOR IS UNABLE TO ASCERTAIN THE REQUIRED CONSTRUCTION OR WHERE CONFLICTS EXIST, IT IS THE CONTRACTOR'S RESPONSIBILITY TO REQUEST ADDITIONAL INFORMATION (RFIs) AND/OR CLARIFICATIONS BEFORE FABRICATION OR CONSTRUCTION.
7. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS BEFORE CONSTRUCTION. THE STRUCTURAL EOR SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES.
8. IT IS THE RESPONSIBILITY OF THE OWNER OR THE OWNER'S AGENT TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES, AS REQUIRED. POSSESSION OF THESE DRAWINGS DOES NOT CONSTITUTE PERMIT APPROVAL.
9. THE GENERAL CONTRACTOR, SUBCONTRACTORS, AND SUPPLIERS SHALL ENSURE COORDINATION OF CONTRACTOR-SUPPLIED/DESIGNED ELEMENTS AND DEFERRED SUBMITTALS WITH ALL DESIGN DISCIPLINES WITHIN THE CONSTRUCTION SET. COORDINATION SHALL IDENTIFY AND RECONCILE CONFLICTS BETWEEN THE CONTRACTOR-SUPPLIED/DESIGNED ELEMENTS AND THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION AND DELIVERY TO THE PROJECT SITE. THE STRUCTURAL EOR SHALL BE NOTIFIED IF CONFLICTS EXIST.
10. WHERE CONFLICTS EXIST BETWEEN STRUCTURAL DOCUMENTS AND OTHER DISCIPLINES, THE STRICTEST REQUIREMENTS, AS INDICATED BY THE STRUCTURAL EOR, SHALL GOVERN.
11. THE CONTRACT STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND ENSURE THE INTEGRITY AND STABILITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION, INCLUDING SHORING AND TEMPORARY SHORING AND BRACING.
12. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN LIVE LOAD FOR THE STRUCTURE. PROVIDE SHORING AND/OR BRACING WHERE LOADS EXCEED DESIGN CAPACITY OR WHERE STRUCTURES HAVE NOT ATTAINED DESIGN STRENGTH.
13. CLADDING, WATERPROOFING, AND ARCHITECTURAL FEATURES ARE OUTSIDE THE STRUCTURAL DESIGN. ANY DEPICTION OF SUCH FEATURES ON THE STRUCTURAL DRAWINGS ARE NOT INTENDED TO BE USED FOR CONSTRUCTION. REPRESENTATION OF SUCH FEATURES ON THESE DRAWINGS MAY OR MAY NOT BE ACCURATE.
14. THE INFORMATION IN THE FOLLOWING GENERAL NOTES SECTIONS OUTLINES KEY REQUIREMENTS BUT IS NOT INCLUSIVE OF ALL RELATED DESIGN, TESTING, REPAIR, AND ACCEPTANCE CRITERIA FOR CONSTRUCTION. SEE THE RELEVANT PROJECT SPECIFICATIONS AND REFERENCED DESIGN STANDARDS FOR ADDITIONAL INFORMATION.

DESIGN LOADS

DESIGN LOADS: PER 2021 IBC AS AMENDED BY THE 2022 OSSC	
MACHINE DEAD LOAD	67,682 LB
PALLET LOADS	11,023 LB (MAX 2)

1603.1.5 - EARTHQUAKE/SEISMIC DESIGN CRITERIA:

RISK CATEGORY	II
SEISMIC IMPORTANCE FACTOR, I _E	1.0
SITE CLASS	E (PER GEOTECH REPORT)
SPECTRAL ACCELERATION, S _S	0.864 G
SPECTRAL ACCELERATION, S ₁	0.415 G
SPECTRAL RESPONSE COEFFICIENT, S _{DS}	0.723 G
SPECTRAL RESPONSE COEFFICIENT, S _{D1}	0.656 G
SEISMIC DESIGN CATEGORY	D
SEISMIC FORCE RESISTING SYSTEM(S)	OTHER MECHANICAL OR ELECTRICAL COMPONENTS (ASCE 7 TABLE 13.6-1)
RESPONSE MODIFICATION FACTORS(S), R	1.5
SEISMIC RESPONSE COEFFICIENTS(S), C _S	0.217, WORKING STRESS
DESIGN BASE SHEAR	19.471 KIPS
ANALYSIS PROCEDURE - EQUIVALENT LATERAL FORCE PROCEDURE, PER AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE) 7	

CONCRETE

1. ALL CONCRETE SHALL BE HARD ROCK CONCRETE MEETING REQUIREMENTS OF ACI-301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE". MIX PROPORTIONS SHALL BE PER ACI-301. SUBMIT MIX DESIGN FOR REVIEW BY STRUCTURAL EOR FOR APPROVAL PRIOR TO CONSTRUCTION.
2. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.
3. ALL STRUCTURAL CONCRETE SHALL BE NORMAL WEIGHT (148 PCF DRY DENSITY, MIN), WITH MIX DESIGNS THAT ATTAIN THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:

TYPE	f _c **	SLUMP*	w/c
FOUNDATION	4,000 PSI	2-4"	0.50

*AVERAGE SLUMP PRIOR TO THE ADDITION OF ANY ADMIXTURE
**SPECIAL INSPECTION NOT REQUIRED. 4,000 PSI COMPRESSIVE STRENGTH IS SPECIFIED FOR DURABILITY. STRUCTURAL DESIGN OF CONCRETE BASED ON 2,500 PSI COMPRESSIVE STRENGTH.

4. PORTLAND CEMENT SHALL BY TYPE I OR II IN CONFORMANCE WITH ASTM C150, OR TYPE IL-MS IN CONFORMANCE WITH ASTM C595 AND ASTM C1012. AGGREGATES SHALL BE IN CONFORMANCE WITH ASTM C33 AND USE CRUSHED (NOT ROUND) GRAVEL OR STONE. COARSE AGGREGATES SHALL NOT EXCEED 1-IN. WATER SHALL BE CLEAN AND POTABLE.
5. CEMENTITIOUS MATERIAL SHALL ONLY BE PORTLAND CEMENT OR ASTM CERTIFIED FLY ASH. UP TO A MAXIMUM OF 15% OF CEMENTITIOUS MATERIAL MAY BE FLY ASH IN ACCORDANCE WITH ASTM C618. BLAST FURNACE SLAG AND OTHER SLAG PRODUCTS ARE NOT ALLOWED. EXCEPTIONS MAY BE USED ONLY WITH PERMISSION OF THE STRUCTURAL EOR.
6. CONCRETE MIXING OPERATIONS, ETC., SHALL CONFORM TO ASTM C94.
7. SLUMP LIMITS MAY BE INCREASED TO A MAXIMUM OF 10" BY ADDITION OF ADMIXTURES PROVIDED THAT THE WATER/CEMENT RATIO OF THE ORIGINAL MIX DESIGN IS NOT EXCEEDED. WATER REDUCING ADMIXTURE SHALL BE IN CONFORMANCE WITH ASTM 494 AND USED IN CONFORMANCE WITH MANUFACTURER INSTRUCTIONS. SUPERPLASTICIZERS MAY BE USED AT THE CONTRACTOR'S OPTION. SUBMIT ADMIXTURES TO STRUCTURAL EOR FOR REVIEW PRIOR TO CONSTRUCTION.
8. REINFORCEMENT BARS SHALL CONFORM TO THE REQUIREMENTS OF THE REINFORCING STEEL AND CONCRETE DETAILING SECTIONS OF THESE GENERAL NOTES.
9. COORDINATE PLACEMENT OF OPENINGS, PIPE PENETRATIONS, CURVES, DOWELS, SLEEVES, CONDUITS, ANCHORS, AND INSERTS PRIOR TO THE PLACEMENT OF CONCRETE. SLEEVES, PIPES OR CONDUITS OF ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE UNLESS EFFECTIVELY COATED.
10. FORMWORK SHALL BE IN ACCORDANCE WITH ACI-347 "GUIDE TO FORMWORK FOR CONCRETE". FORMS SHALL BE DESIGNED BY THE CONTRACTOR. SHORING AND TEMPORARY SUPPORTS SHALL BE PROVIDED AS REQUIRED OR UNTIL THE CONCRETE HAS REACHED ITS SPECIFIED 28-DAY STRENGTH. FORMWORK, SHORING, AND TEMPORARY SUPPORTS SHALL PROVIDE FINISHED CONCRETE SURFACES AT ALL FACES: LEVEL, PLUMB, AND TRUE TO DIMENSIONS AND ELEVATIONS SHOWN IN THE DRAWINGS.
11. CHAMFER ALL EXTERIOR CORNERS 1/2-IN UNLESS SHOWN OTHERWISE.
12. TOLERANCES FOR CONCRETE FORMWORK SHALL CONFORM TO AMERICAN CONCRETE INSTITUTE (ACI) 117. STANDARD TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS, UNLESS OTHERWISE SPECIFIED.
13. CONSTRUCTION MATERIALS SHALL BE UNIFORMLY SPREAD OUT SUCH THAT DESIGN LIVE LOAD PER SQUARE FOOT AS NOTED HEREIN IS NOT EXCEEDED.
14. MECHANICALLY VIBRATE ALL FORMED CONCRETE. DO NOT OVER-VIBRATE. PLACE CONCRETE MONOLITHICALLY BETWEEN CONSTRUCTION OR CONTROL JOINTS. PROTECT ALL CONCRETE FROM PREMATURE DRYING.
15. TOOL SLAB JOINTS AT THE TIME OF FINISHING. SAW CUTTING IN NOT ALLOWED UNLESS APPROVED BY THE STRUCTURAL EOR.
16. WHERE NEW CONCRETE IS PLACED AGAINST EXISTING CONCRETE, THE EXISTING CONCRETE SURFACE SHALL BE CLEANED AND ROUGHENED TO A MINIMUM 1/4-IN AMPLITUDE PER ACI 318.
17. CONSTRUCTION JOINTS INDICATED ARE SUGGESTED LOCATIONS. CONTRACTOR MAY REVISE LOCATION OF JOINTS. SUBJECT TO SPECIFIED REQUIREMENTS. LAYOUT SHOWING ALL CONSTRUCTION JOINT LOCATIONS SHALL BE SUBMITTED FOR REVIEW BY STRUCTURAL EOR.

FOUNDATIONS

1. GEOTECHNICAL REPORT, DATED SEPTEMBER 27, 2007 (THEIR FILE NO. 092707), WAS PREPARED BY GEODESIGN GEOTECHNICAL, INC. OF SCAPPOOSE, OREGON. THE CONTRACTOR SHALL BE FAMILIAR WITH THAT SUPPLEMENTAL REPORT INFORMATION AND THE RECOMMENDATIONS CONTAINED THEREIN.
2. ALL FOUNDATIONS TO BEAR ON UNDISTURBED NATIVE MATERIAL, OR GRANULAR COMPACTED ENGINEERED FILL, PER THE PROJECT CONTRACT DOCUMENTS. THE CONTRACTOR IS DIRECTED TO THE GEOTECHNICAL REPORT IN THE PROJECT SUPPLEMENTAL INFORMATION FOR ADDITIONAL INFORMATION. EXCAVATIONS FOR FOUNDATIONS SHALL BE OBSERVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING OF CONCRETE FOR FOUNDATION.
3. SOIL DESIGN CRITERIA, PER GEOTECHNICAL ENGINEER:

3.1. SOIL BEARING - 2000 PSF

3.1.1. 1/3 INCREASE ALLOWED FOR SHORT TERM LOADS

3.2. FRICTION COEFFICIENT - 0.30
4. LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN IN SERVICE DURING AND/OR AFTER CONSTRUCTION.

CONCRETE DETAILING

1. DETAIL AND PLACE REINFORCING STEEL ACCORDING TO AMERICAN CONCRETE INSTITUTE (ACI) 315.
2. UNO. MINIMUM COVER SHALL BE 1 1/2-IN FOR #5 AND SMALLER BARS, 2-IN FOR #6 AND LARGER BARS AND 3-IN WHEN POURED AGAINST EARTH. ALL REINFORCING STEEL SHALL BE SUPPORTED ON STANDARD APPROVED ACCESSORIES SUCH AS CHAIRS, SPACERS OR TIES, HELD RIGIDLY AND ACCURATELY IN PLACE, AND PROTECTED AGAINST DISPLACEMENT BEFORE AND DURING THE PLACEMENT OF CONCRETE.
3. ALL LAPS. UNO. SHALL BE MINIMUM 48 BAR DIAMETERS AT SPLICES AND NOT LESS THAN 24-IN. NO MORE THAN 50% OF REINFORCING SHALL BE SPLICED AT ANY LOCATION. UNO. ALL HORIZONTAL REINFORCING AT WALL/FOOTING CORNERS SHALL BE BENT BAR WITH MINIMUM EMBEDMENT BEYOND INTERFACE PER THE DEVELOPMENT LENGTH SPECIFIED IN ACI 318 (MINIMUM 2-FT LAP LENGTH).
4. SPLICES SHALL BE MADE ONLY WHERE INDICATED ON THE STRUCTURAL DRAWINGS.

4.1. SPLICES IN CONTINUOUS TOP BARS, IF REQUIRED, SHALL OCCUR OVER THE CENTER OF THE OPENING SPAN OR AT LEAST 2X THE LAP LENGTH AWAY FROM SUPPORTS.

4.2. SPLICES IN CONTINUOUS BOTTOM BARS, IF REQUIRED, SHALL OCCUR OVER SUPPORTS OR CENTERED OVER COLUMNS.

4.3. SPLICES SHALL BE CONTACT LAP SPLICES.

CAST-IN-PLACE ANCHORS - CONCRETE & MASONRY

1. CONTRACTOR SHALL CONTACT THE STRUCTURAL EOR IF CAST-IN-PLACE ANCHORS ARE MISSING OR MISPLACED AFTER THE CONCRETE POUR.
2. HOOKED, HEADED, THREADED, OR NUTTED ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 36 (FY = 36 KSI), UNO.
3. CAST-IN-PLACE ANCHORS SHALL BE OF THE TYPE AND PRODUCT SPECIFIED ON THE DRAWINGS. REQUESTS FOR ANCHOR SUBSTITUTIONS SHALL BE SUBMITTED TO THE STRUCTURAL EOR IN WRITING ALONG WITH EVIDENCE OF EQUAL OR GREATER CAPACITY FOR THE SPECIFIED CONNECTION PRIOR TO CONCRETE/GROUT PLACEMENT.
4. CLEAN DIRT, RUST, OIL, AND LOOSE PAINT/COATINGS FROM ANCHORS PRIOR TO INSTALLATION. THREADS ON THE PROJECTING PORTION OF THE ANCHOR ELEMENT SHALL BE PROTECTED FROM CONTAMINATION.
5. INSTALL ANCHORS IN ACCORDANCE WITH THE SPACING AND EDGE DISTANCES INDICATED ON THE DRAWINGS, BUT NOT LESS THAN THE FOLLOWING AMERICAN CONCRETE INSTITUTE (ACI) 318 17.9 CODE MINIMUMS:

5.1. MINIMUM CENTER TO CENTER ANCHOR SPACING OF:

5.1.1. SIX TIMES THE ANCHOR DIAMETER (6"DA)

5.2. MINIMUM EDGE DISTANCE MEASURED FROM ANCHOR CENTER LINE TO CONCRETE EDGE OF:

5.2.1. EQUAL TO THE REINFORCING CLEAR COVER REQUIREMENTS OUTLINED IN THE CONCRETE SECTION OF THESE GENERAL NOTES FOR ANCHORS THAT WILL NOT BE TORQUED

5.2.2. SIX TIMES THE ANCHOR DIAMETER (6"DA) FOR ANCHORS THAT WILL BE TORQUED

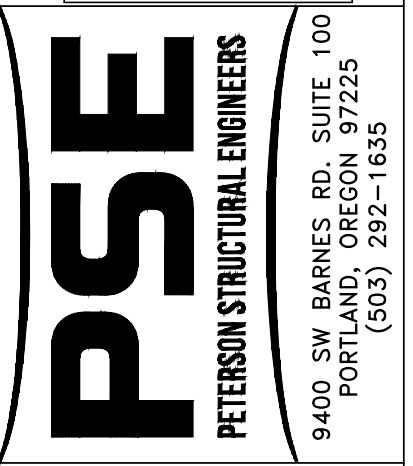
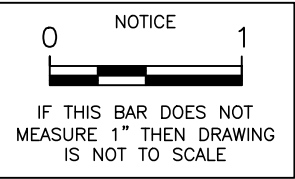
5.3. IF CONFLICT IS IDENTIFIED BETWEEN THE MANUFACTURER RECOMMENDED MINIMUM EDGE DISTANCES AND/OR SPACING AND THOSE INDICATED ON THESE DRAWINGS CONTACT THE STRUCTURAL EOR.
6. CONTACT BETWEEN DISSIMILAR METALS SHALL BE ISOLATED USING PHENOLIC OR OTHERWISE APPROVED ISOLATION HARDWARE. ANCHORS EXPOSED TO EARTH OR WEATHER SHALL BE PROTECTED FROM CORROSION PER THE FOLLOWING REQUIREMENTS, UNO.

6.1. ANCHORS SHALL BE HOT-DIPPED GALVANIZED OR STAINLESS FOR FASTENING GALVANIZED STEEL TO CONCRETE/MASONRY.

6.2. ANCHORS SHALL BE STAINLESS STEEL FOR FASTENING ALUMINUM OR STAINLESS STEEL TO CONCRETE/MASONRY.
7. NUTS, WASHERS, AND OTHER HARDWARE USED WITH CAST-IN-PLACE ANCHORS SHALL HAVE A MATERIAL OR ALLOY DESIGNATION THAT IS COMPATIBLE WITH THE ANCHOR ROD/ALLOY.
8. CAST-IN-PLACE ANCHORS SHALL NOT BE BENT AFTER BEING INSTALLED UNLESS PERMITTED BY THE STRUCTURAL EOR.
9. INDEPENDENT ON-SITE PROOF LOAD TESTING SHALL BE PERFORMED AS REQUIRED BY THE STRUCTURAL EOR. CONTACT STRUCTURAL EOR FOR NUMBER OF ANCHORS REQUIRED TO BE TESTED AND REQUIRED PROOF LOAD MAGNITUDE.

REINFORCING STEEL

1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60.
2. REINFORCING STEEL SHALL NOT BE TACK WELDED FOR ANY REASON. WELDED REINFORCING BARS WILL ONLY BE APPROVED IF EXPLICITLY SHOWN ON THE STRUCTURAL DRAWINGS OR IF WRITTEN APPROVAL IS GIVEN BY THE STRUCTURAL EOR.
3. DOWELS AND OTHER MISCELLANEOUS STEEL EMBEDDED ITEMS SHALL BE LOCATED AND HELD IN SPECIFIED POSITION PRIOR TO PLACEMENT OF CONCRETE/GROUT AND SHALL NOT BE PUSHED INTO CONCRETE/GROUT FOLLOWING CONCRETE/GROUT POUR.
4. ALL REINFORCING BENDS SHALL BE MADE COLD. BARS SHALL NOT BE UN-BENT AND RE-BENT. FIELD BENDING OF REBAR SHALL NOT BE ALLOWED UNLESS SPECIFICALLY NOTED. NO BARS PARTIALLY EMBEDDED IN HARD CONCRETE/GROUT SHALL BE FIELD BENT UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE STRUCTURAL EOR.
5. DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME GRADE, SIZE, SPACING AND NUMBER AS THE SPECIFIED VERTICAL REINFORCING UNO.



OMIC BUILDING 1 HM 1000 FOUNDATION

SHEET CONTENT
GENERAL STRUCTURAL
NOTES

JOB No.
2401-0097

DRAWN CTN
CHECKED EFL

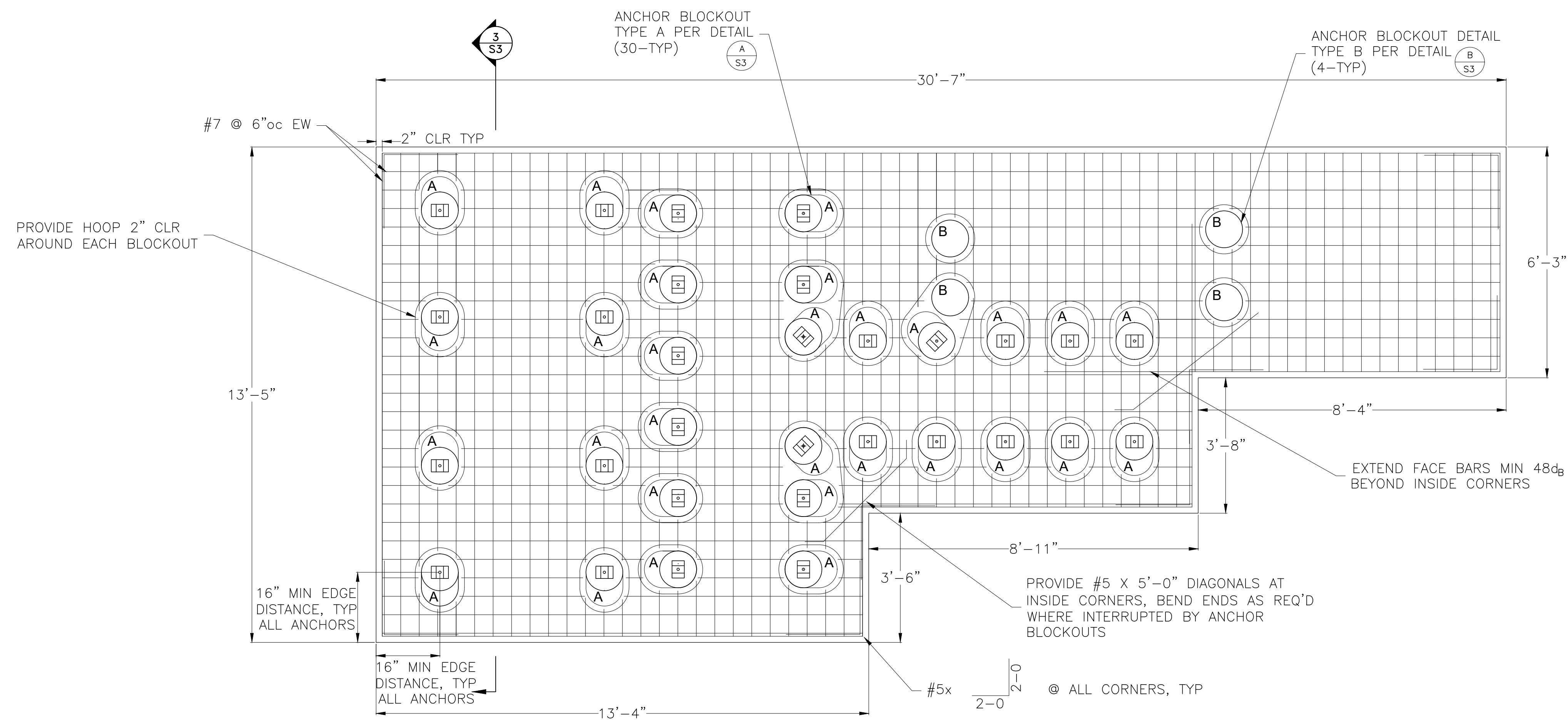
DATE 02/27/25

REVISIONS

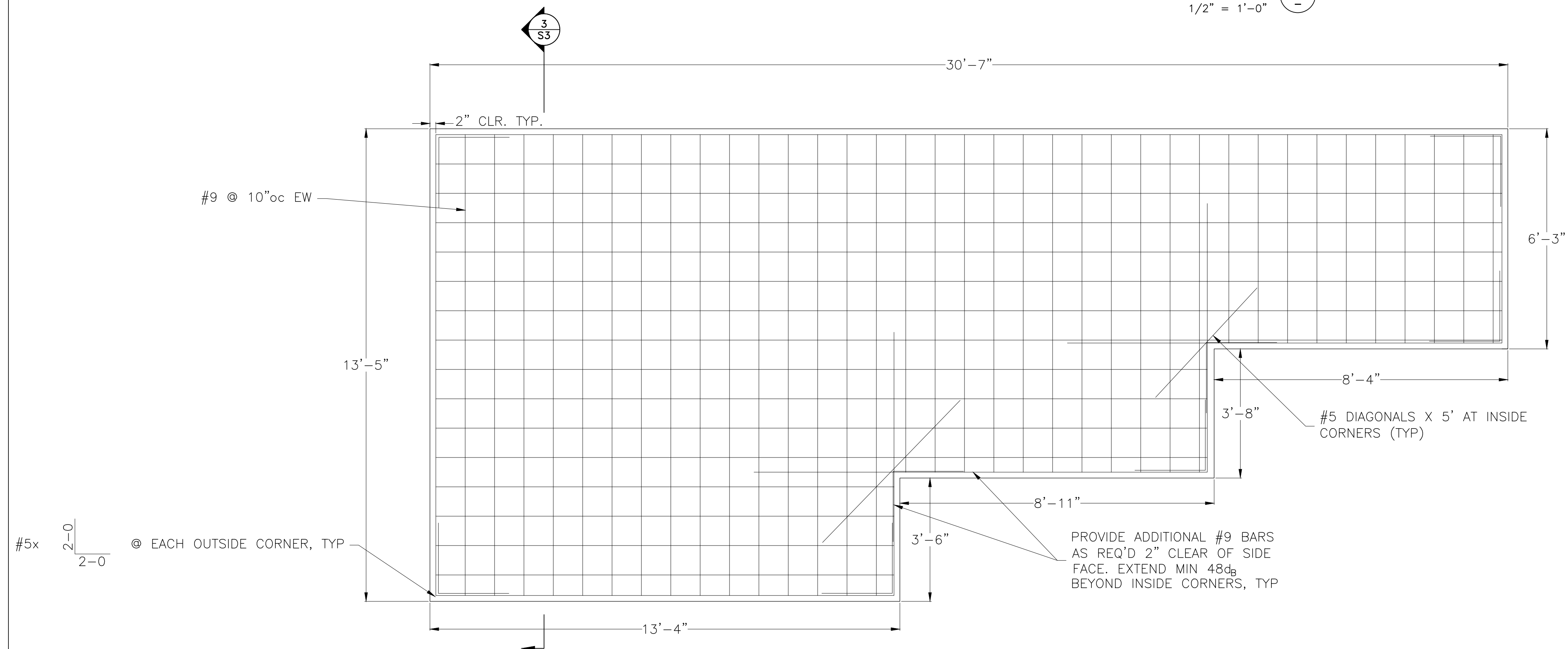
SHEET
S1 of 3

CLIENT INFO:
AKKAAN ARCHITECTURE & DESIGN
101 ST. HELENS ST.
ST. HELENS, OR 97051

PROJECT SITE:
33701 CHARLES T PARKER WAY
SCAPPOOSE, OR 97056



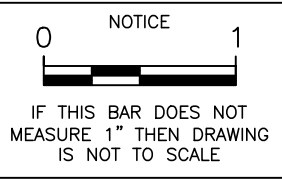
FOUNDATION PLAN – ANCHOR LAYOUT & TOP REINFORCING



FOUNDATION PLAN - BOTTOM REINFORCING

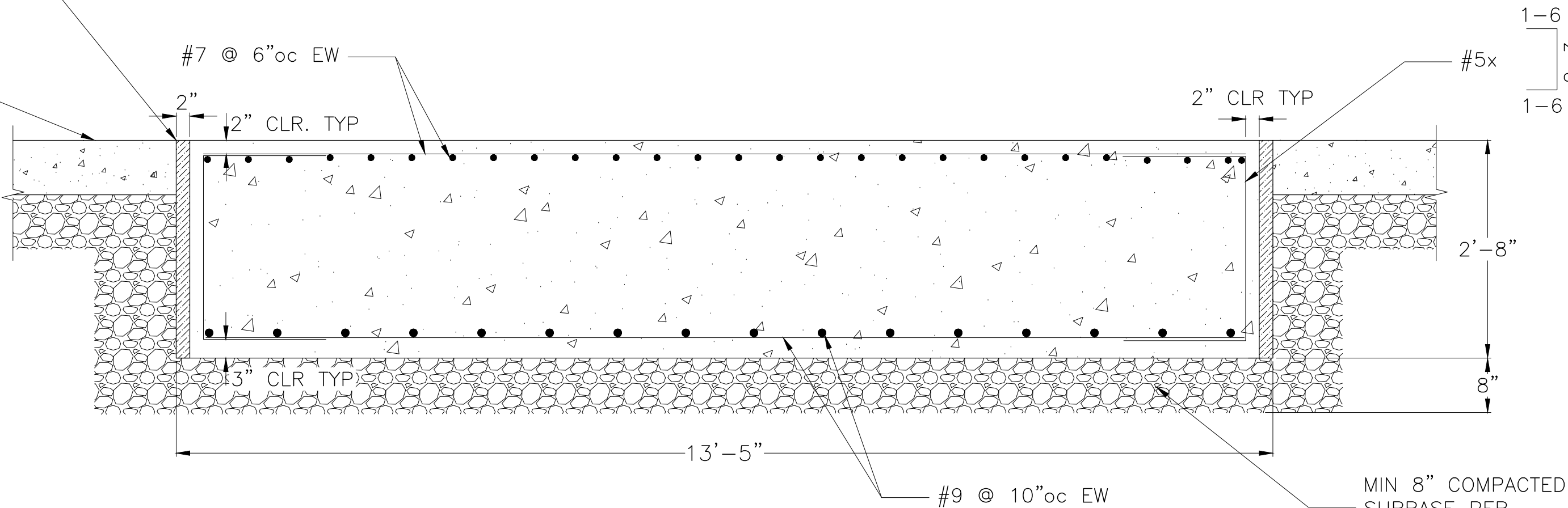
- CONSTRUCTION NOTES:

1. ANCHOR LOCATIONS ARE APPROXIMATE. COORDINATE WITH MANUFACTURER FOR EXACT LOCATIONS. NOTIFY ENGINEER OF RECORD IF POSITIONS DEVIATE BY MORE THAN 6"
2. ANCHORS MUST MAINTAIN MINIMUM 16" EDGE DISTANCE TO EDGE OF FOUNDATION SLAB.
3. DISCONTINUE TOP REINFORCING BARS AT ANCHOR BLOCKOUTS.
4. MAINTAIN 1" CLEAR SPACING BETWEEN REINFORCING AND ANCHOR BLOCKOUTS.
5. BAR POSITIONS TO MAINTAIN #7@6"oc AVERAGE SPACING, INDIVIDUAL BARS MAY BE ADJUSTED UP TO 1.5" TO CLEAR BLOCKOUT AS REQUIRED. MAINTAIN MINIMUM 2" COVER FROM SIDE FACES.
6. WHERE FACE BARS ALIGN WITH GRID REINFORCEMENT, ADDITIONAL FACE BARS NOT REQUIRED.



PROVIDE 2" ISOLATION GAP
BETWEEN FOUNDATION AND (E)
SLAB PER CONSTRUCTION NOTES

(E) 8" BUILDING SLAB.
REINFORCING NOT SHOWN



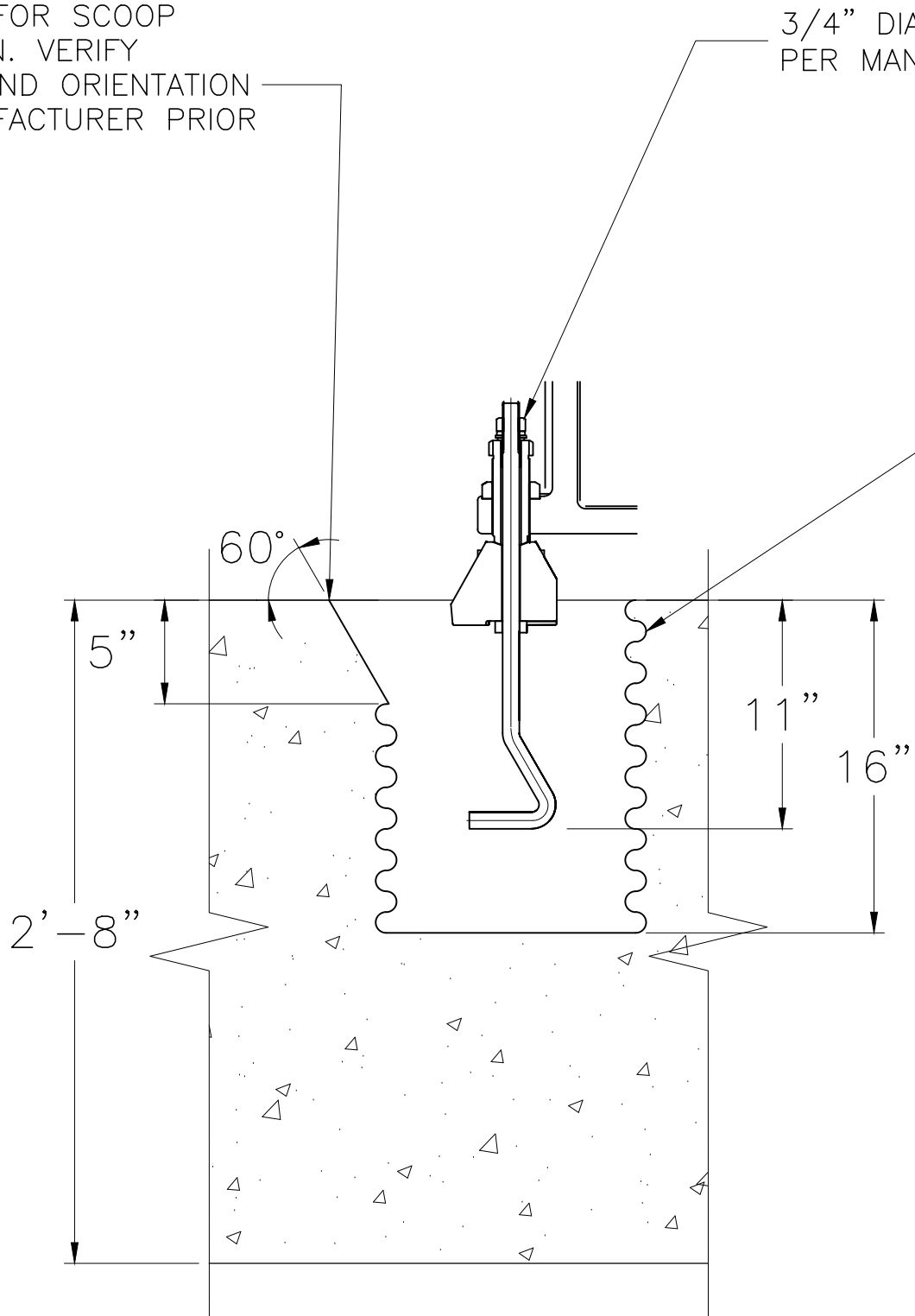
FOUNDATION SECTION 3
3/4" = 1'-0"

- CONSTRUCTION NOTES:
1. SAWCUT EXISTING SLAB TO PROVIDE ADDITIONAL 2" BEYOND MACHINE FOUNDATION FOOTPRINT. DO NOT OVERCUT AT CORNERS. CUT SHORT AND CHIP OUT CORNERS.
 2. APPLY CORROSION INHIBITOR TO CUT METAL SURFACES OF EXISTING SLAB. SIKa EPOCEM 110 OR APPROVED EQUAL.
 3. EXERCISE CAUTION WHEN EXCAVATING NOT TO UNDERMINE EXISTING 8" SLAB SUBGRADE.
 4. PROVIDE MIN 8" SUBBASE LAYER OVER COMPENT NATIVE SUBGRADE. USE WELL-GRADED, 1-1/2" MINUS CLEANED CRUSHED ROCK OR GRAVEL COMPACTED TO 95% MAXIMUM DRY DENSITY PER ASTM D1557.
 5. PROVIDE 2" ISOLATION GAP ALL AROUND FOUNDATION W/ COMPRESSIBLE JOINT FILLER (WR MEADOWS X-FOAM OR APPROVED EQUAL). SEAL W/ ELASTOMERIC CAULK OR SEALANT COMPATIBLE WITH JOINT FILLER.

SEE PLAN FOR SCOOP
ORIENTATION. VERIFY
LOCATION AND ORIENTATION
WITH MANUFACTURER PRIOR
TO POUR

3/4" DIA. ANCHOR,
PER MANUFACTURER

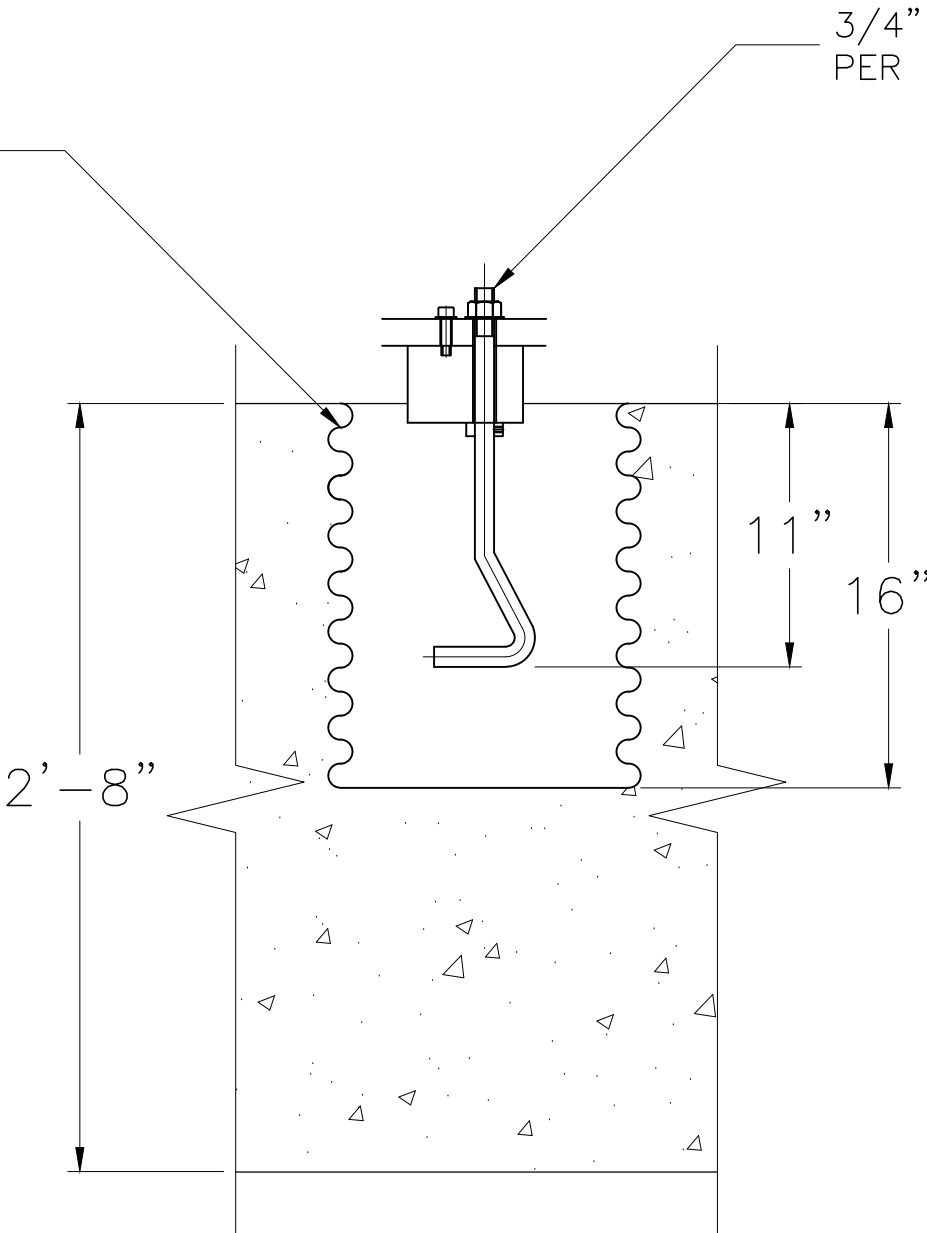
12" DIA. 26 GAUGE
CORRUGATED METAL PIPE
BLOCKOUT. REMOVE PRIOR
TO GROUTING



ANCHOR DETAIL - TYPE A A
1 1/2" = 1'-0"

12" DIA. 26 GAUGE
CORRUGATED METAL PIPE
BLOCKOUT. REMOVE PRIOR
TO GROUTING

3/4" DIA. ANCHOR,
PER MANUFACTURER



ANCHOR DETAIL - TYPE B B
1 1/2" = 1'-0"