

BUILDER / CONTRACTOR RESPONSIBILITIES

It is the responsibility of the builder/contractor to insure that all project plans and specifications comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that the building manufacturer or its design engineer is acting as the engineer of record or design professional for a construction project. The contractor must secure all required approvals and permits from the appropriate agency as required. Approval of the manufacturer's drawings and calculations indicate that the building manufacturer correctly interpreted and applied the requirements of the contract drawings and specifications. (Sect. 4.2.1 AISC Code of Standard Practices, 9th Ed.) Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (Sect. 3.3 AISC Code of Standard Practice 9th Ed.) Design considerations of any materials in the structure which are not furnished by the building manufacturer are the responsibility of the contractors and engineers other than the building manufacturer's engineers unless specifically indicated. The contractor is responsible for all erection of steel and associated work in compliance with the building manufacturer's "For Erector Installation" drawings. Products shipped to builder or his customer shall be inspected by builder immediately upon arrival. Claims for shortages or defective material if not packaged must be mailed to the manufacturer in writing within five (5) days after receipt of the shipment. However, if a defect is of such a nature that reasonable visual inspection would fail to disclose it, then the claim must be made within five (5) days after the builder learns of the defect. The manufacturer will not be liable for any defect unless claim is made within one (1) year after date of the original shipment by the manufacturer to builder or his customer. The manufacturer will be given a reasonable opportunity to inspect defective materials upon receipt of claim by builder. If a defect is of such nature that it can be remedied by a field operation at the job site without the necessity of returning the material to the manufacturer, then upon written authorization of the manufacturer the builder may repair or cause the material to be repaired and the manufacturer will reimburse the builder for the cost of the repair in accordance with the written authorization. Unless noted otherwise, all bracing as shown and provided by the manufacturer for this building is required and shall be installed by the erector as a permanent part of the structure. Temporary supports, such as temporary guys, braces, false work, cribbing or other elements required for the erection operation will be determined and furnished and installed by the erector. These temporary supports will secure the steel framing, or any partly assembled steel framing, against loads comparable in intensity to those for which the structure was designed, resulting from wind, seismic forces and erection operations, but not the loads resulting from the performance of work by or the acts of others, nor such unpredictable loads as those due to tornado, explosion or collision. (Sect. 7.9.1 AISC Code of Standard Practice, 9th Ed.) Design of gutter and downspout is a function of the rainfall intensity and area to be drained. Design parameters utilized are in accordance with the 2002 low rise building systems manual and/or the 9th Edition of the architectural graphic standards, as applicable. Proper owner maintenance dictates that the drainage system be kept free and clear of debris and/or ice at all times to ensure proper function of the gutter and downspout. In those cases where the owner/tenant of a property is unwilling or unable to provide proper maintenance, elimination of gutter should be considered as an alternative.

PRODUCT CERTIFICATIONS

The building manufacturer is a member of the Metal Building Manufacturers Association.

The building manufacturer's fabrication and products are covered by one or more of the following certifications:

1. Approved fabricator of prefabricated buildings and components. Reference IAS (FA-337).
2. SBCCI compliance report no. 94610
3. AISC category MB.
4. City of Houston approved fabricator (registration no. 164)
5. Wisconsin product approval number 200231-M
6. Clark County, Nevada approved fabricator
7. City of Los Angeles, California approved type 1 fabricator (LA#604)
8. Canadian Welding Bureau certification to CSA standard W47.1 in division 1 (Houston, TX)
9. Texas Dept. of Insurance product evaluation RC-15

APPROVAL NOTES

The following conditions apply in the event that these drawings are used as approval drawings:

- A) It is imperative that any changes to these drawings:
 - 1) Be made in contrasting ink.
 - 2) Have all instances of change clearly indicated.
 - 3) Be legible and unambiguous.
- B) Dated signature is required on all pages.
- C) Manufacturer reserves the right to re-submit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.
- D) Approval of these drawings indicates conclusively that the manufacturer has correctly interpreted the contract requirements, and further constitutes agreement that the building as drawn, or as drawn with indicated changes represents the total of the materials to be supplied by manufacturer.
- E) Any changes noted on the drawings not in conformance with the terms and requirements of the contract between manufacturer and its customer are not binding on manufacturer unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation. Manufacturer recognizes that rubber stamps are routinely used for indicating approval, disapproval, rejection, or mere review of the drawings submitted. However, manufacturer does not accept changes or additions to contractual terms and conditions that may appear with use of a stamp or similar indication of approval, disapproval, etc. Such language applied to manufacturer's drawings by the customer, architect, engineer, or any other party will be considered as unacceptable alterations to these drawing notes, and will not alter the contractual rights and obligations existing between manufacturer and its customer.

BOLT TIGHTENING

The proper tightening and inspection of all fasteners is the responsibility of the Erector. All high strength (A325, A490) bolts and nuts must be tightened by the "Turn-of the nut" method unless otherwise specified by the End Customer in the Contract Documents, or as allowed by the provisions below. Inspection of high strength bolt and nut installation by other than the Erector must also be specified in the Contract Documents and the Erector is responsible for ensuring that the installation and inspection procedures are compatible prior to the start of erection. (MEMA '96 IV 6.6)

According to AISC and RCSC, as long as the bolt holes are not oversized or slotted, snug-tightened bolts, (A325 only), are permitted except in buildings supporting cranes exceeding a 5 ton capacity or in buildings located in Seismic Design Categories D and greater. It is important to check with the local building authority on the use of "snug-tightened" bolts since the model code groups (BOCA, SBCCI, and ICBO), have not yet formally adapted this change.

SAFETY COMMITMENT

The building manufacturer has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of the building manufacturer. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, state and federal safety and health standards, whether standard statutory or customary, should always be followed to help insure worker safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended. For purposes of determining lift requirements, no bundle supplied by NCI will exceed 4,000 pounds. For further information also reference the bill of materials for individual member weights of other structural members. If additional information is required contact the customer service department at 800-777-9378.

GENERAL NOTES

- The structure under this contract has been designed and detailed for the loads and conditions stipulated in the contract and shown on these drawings. Any alterations to the structural system or removal of any component parts, or the addition of other construction materials or loads must be done under the advice and direction of a registered architect, civil or structural engineer. The building manufacturer will assume no responsibility for any loads not indicated. This metal building is designed with the building manufacturer's standard practices which are based on pertinent procedures and recommendations of the following organizations and codes.
1. American Institute of Steel Construction: "Specification for the Design, Fabrication and Erection of Structural Steel for Buildings"
 2. American Iron and Steel Institute: "Specification for the Design of Cold Formed Steel Structural Members"
 3. American Welding Society: "Structural Welding Code" AWS D1.1.
 4. American Welding Society: "Structural Welding Code" AWS D1.3
 5. Metal Building Manufacturer's Association: "Low Rise Building Systems Manual"
 6. International Conference of Building Officials: "Uniform Building Code"
 7. Southern Building Code Congress International: "Standard Building Code"
 8. Building Official and Code Administrators International: "Booca National Building Code"
 9. National Building Code of Canada.
 10. International Building Code (IBC)

Material properties of steel plate used in the fabrication of primary rigid frames, and other primary structural exclusive of cold-formed sections, conform to ASTM-A529 or A-572. Flanges with thickness of 1" inch or less and width of 12" or less conform to A-529 with a minimum yield point of 55,000 PSI. Flanges greater than 3/8" in thickness and 12" in width conform to A-572 with a minimum yield point of 50,000 PSI. Flanges with a thickness greater than 1" thick and a width less than 12" conform to A-572 with a minimum yield point of 50,000 PSI. Web material less than 1/4" thick conforms to ASTM-A1011 with a minimum yield point of 50,000 PSI. Web material 1/4" thick and greater conform to A-572 with a minimum yield point of 50,000 PSI. Material properties of pipe sections conform to ASTM-A53 type E, Grade B with a minimum yield point of 35,000 PSI. Material properties of hot rolled steel members conform to the requirements of ASTM-A992 or A-572 with a minimum yield point of 50,000 PSI. Material properties of cold formed light gage steel members conform to ASTM-A1011 Grade 55 modified with a minimum yield point of 57,000 PSI. Material properties of roof/wall sheeting, base metal conform to ASTM-A792 Grades 50 or 80 with minimum yield points of 50,000 PSI and 80,000 PSI respectively, as required by design. Coating of base material is 55% aluminum-zinc alloy in accordance with AZ55 for unpainted or AZ50 for painted specifications. Cable utilized for bracing conforms to ASTM A475. Cable bracing is to be installed to a taut condition with all slack removed. Rod and angle utilized for bracing members conform to ASTM A36. Structural joints with ASTM A-325 high strength bolts, where indicated on the drawings, shall be assembled and the fasteners tightened in accordance with the bolt tightening procedure indicated below. All joints will be assembled without washers unless otherwise noted. All steel members except bolts, fasteners and cable shall receive one shop coat of iron oxide corrosion inhibitive primer, meeting the performance requirements of SSPC Paint Specification #15. Shop and field inspections and associated fees are the responsibility of the contractor, unless stipulated otherwise in the contract.

ROOF MAINTENANCE GUIDELINES

- * Walk in the flat of the panels near the structural supports.
- * Keep roof, gutters and downspouts free of debris.
- * Inspect roof for damage after heavy storms.
- * Remove excessive ice and snow accumulations as necessary.
- * Inspect and reset as necessary all roof curbs and other penetrations with urethane sealant.
- * Always get manufacturer approval before making any modifications to the roof.
- * Repaint any areas that are susceptible to rust as required.

When performing roof maintenance, always take the following precautions:

- * Use fall protection and other safety equipment as required.
- * Do not walk on roof flashings such as gutter, rake, hip or ridge flash.
- * Do not walk on Light Transmitting Panels (LTPs). They will not support a person's weight.
- * Guard all LTPs and roof openings.
- * Step only in the panel flat directly on or in close proximity to a supporting roof structural.
- * After a fire, vandalism or known damage to an adjacent roof area.
- * Exposure to severe weather conditions, including high winds, hail or abnormally heavy rains or ice and snow.
- * After other trades have been on the roof for any reason, inspect the roof for damage caused by workers including chemical or solvent spills, scratches in the paint or Galvalume® coating, excessive foot traffic and punctures. Make sure that any debris or scrap left behind by the workers is removed from the roof immediately. Avoid using cutoff saws and welding equipment over the roof. In cases where this is not possible, the roof must adequately protected.

FOOT TRAFFIC

Keep foot traffic to a minimum. Heavy foot traffic can cause ponding on low pitched roofs. This is particularly true just uplope from the eave and at endlaps. Always walk in the flat of the panel near a supporting roof structure. Do not walk on trim or in gutters. On bare Galvalume® roofs, excessive foot traffic may cause black burnish marks. If regular foot traffic is planned for a roof, provisions should be made for a properly designed and installed roof walkway system. In order to limit access to the roof, roof hatches or access ladders should be locked at all times. A sign should be posted at the point of access, stating that only authorized personnel are allowed onto the roof. In addition, a log book should be kept of all visits to the roof and the reason for such visits.

DRAINAGE

- * Keep roof free of debris and keep debris out of gutter to allow water to quickly drain from roof.
- * Do not use wood blocking to hold equipment off of panel seams. This blocks the flow of water and holds moisture.
- * Do not allow rooftop AC units or evaporative coolers to drain onto the roof.
- * Anything that traps or holds moisture on a roof will cause premature corrosion.

POST-ERECTION ROOF CLEANING

After roof installation is complete, all filings, shavings, etc. from fastener installation etc. shall be swept completely clear of the roof panels. If this does not happen, the manufacturer will not be held responsible for any resulting corrosion.

ICE AND SNOW REMOVAL

Excessive ice and snow should be removed from roof immediately to prevent damage to roof and possible collapse. Do not use metal tools to remove the ice or snow as this can damage the paint and/or Galvalume coatings. Also, be careful around plumbing pipes and flashings. Be extremely careful if your roof has Light Transmitting Panels. THESE PANELS WILL NOT SUPPORT A PERSON'S WEIGHT AND WILL BE DIFFICULT OR IMPOSSIBLE TO SEE IF THEY ARE COVERED WITH ICE AND SNOW. See 2002 MEMA Low-Rise Building Systems Manual appendix a8 for details snow removal procedures. THESE PROCEDURES SHOULD COMMENCE WHEN HALF OF THE DESIGN ROOF SNOW LOAD SHOWN ON THIS SHEET IS REALIZED.

DRAWING STATUS

	ISSUE	DATE	DESCRIPTION	BY	CK'D
☐	For Approval:	A	8/31/07	FOR CONSTRUCTION PERMIT	JAC DSW
☐	For Construction Permit:	B	9/14/07	REVISED FOR CONSTRUCTION PERMIT	JAC BB
☐	For Erector Installation:				

ISSUE	DATE	DESCRIPTION	BY	CK'D

METALLIC

metallic building company

7301 FAIRMW, HOUSTON, TEXAS, P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Project: TRUCK SHOP / OFFICE

Customer: HEDGES CREEK MANAGEMENT # 1 LLC Owner: HEDGES CREEK MANAGEMENT # 1 LLC

Location: SCAPPOUSE, OR

CAD Date Scale Phase Building ID Job Number Sheet Number Issue

N.T.S. - A 0805-236857 C1 OF 1 B

8164

09/14/07

OREGON

JULY 22, 1971

ERT S. HOEKSTRA

9164

09/14/07

OREGON

JULY 22, 1971

ERT S. HOEKSTRA

EXPIRES 09/30/09

BUILDING DESCRIPTION:

BUILDING DESCRIPTION:					Endwall Frame Type	
Building D	Width	Length	Height	Roof Pitch	Left	Right
BLDG. A	CST 115'-0	240'-0	23'-0	1: 12	NON-EXP MAIN FRAME	NON-EXP MAIN FRAME
					-	-

WARNING: In no case should Galvalume steel panels be used in conjunction with lead, copper, Portland Cement, or treated lumber. These materials have harmful corrosive effects on the aluminum zinc alloy coating when they are used in contact with Galvalume steel panels. Even run-off from these materials including flashing, wiring, or tubing onto Galvalume should be avoided.

Base Condition	Sheeting			Blanket Type Insulation						
NO SHEETING RECESS W/	24 Ga.	***DOUBLE-LOK/ GALVALUME	Roof	None	By Manufacturer	By Others	X	Roof	4" Wall	4" UL-25
BASE ANGLE/ FLASH	26 Ga.	PSR/ ASH GRAY	Wall	Screws: SELF DRILLING						
Tape Seal			Trim (1/8" Pop rivets at splices)	Member Roof	***	Stitch Roof	***	Anchor Bolts		
3/8" x	26 Ga.	POLAR WHITE	Rake	Member Wall	**12-14 x 1 1/4 SDS	Stitch Wall	**14 x 7/8 Laplok	By Others X		
1" x	26 Ga.	POLAR WHITE	Eave	Rake to Roof	STITCH	Rake to Wall	STITCH	By Building Manufacturer		
Warranties			28 Ga.	POLAR WHITE	Gutter	Gutter to Roof	STITCH	Gutter Straps	STITCH	
U.L. 90	NO	26 Ga.	ASH GRAY	Downspouts	Corner Trim	STITCH	Rake Angle	MEMBER		
20 Yr Roof	YES	26 Ga.	ASH GRAY	Corner	Primary X	Red	Gray	Golv.		
20 Yr Wall	YES	26 Ga.	POLAR WHITE	Accessories	Secondary	X	Red	Gray	Golv.	
Additional Features - **DENOTES LONG LIFE FASTENERS *** REFERS TO DOUBLE-LOK MANUAL W/ LOW FLOATING CLIPS										

BUILDING LOADS

Applicable Interior Zone FM Rating I- Perimeter & Corner Zones per FM Data Sheet I-28, Table 6.

This is to certify that this structure is designed utilizing the loads indicated and applied as required by the building code shown below. This certification is limited to the structural design of the framing and covering parts manufactured by the building manufacturer and as specified in the contract. Accessory items such as doors, windows, louvers, translucent panels, and ventilators are not included. Also excluded are other parts of the project not provided by the building manufacturer such as foundations, masonry walls, mechanical equipment and the erection of the building. The building should be erected on a properly designed foundation in accordance with the building manufacturer's design manual, the attached drawings, and good erection practices.

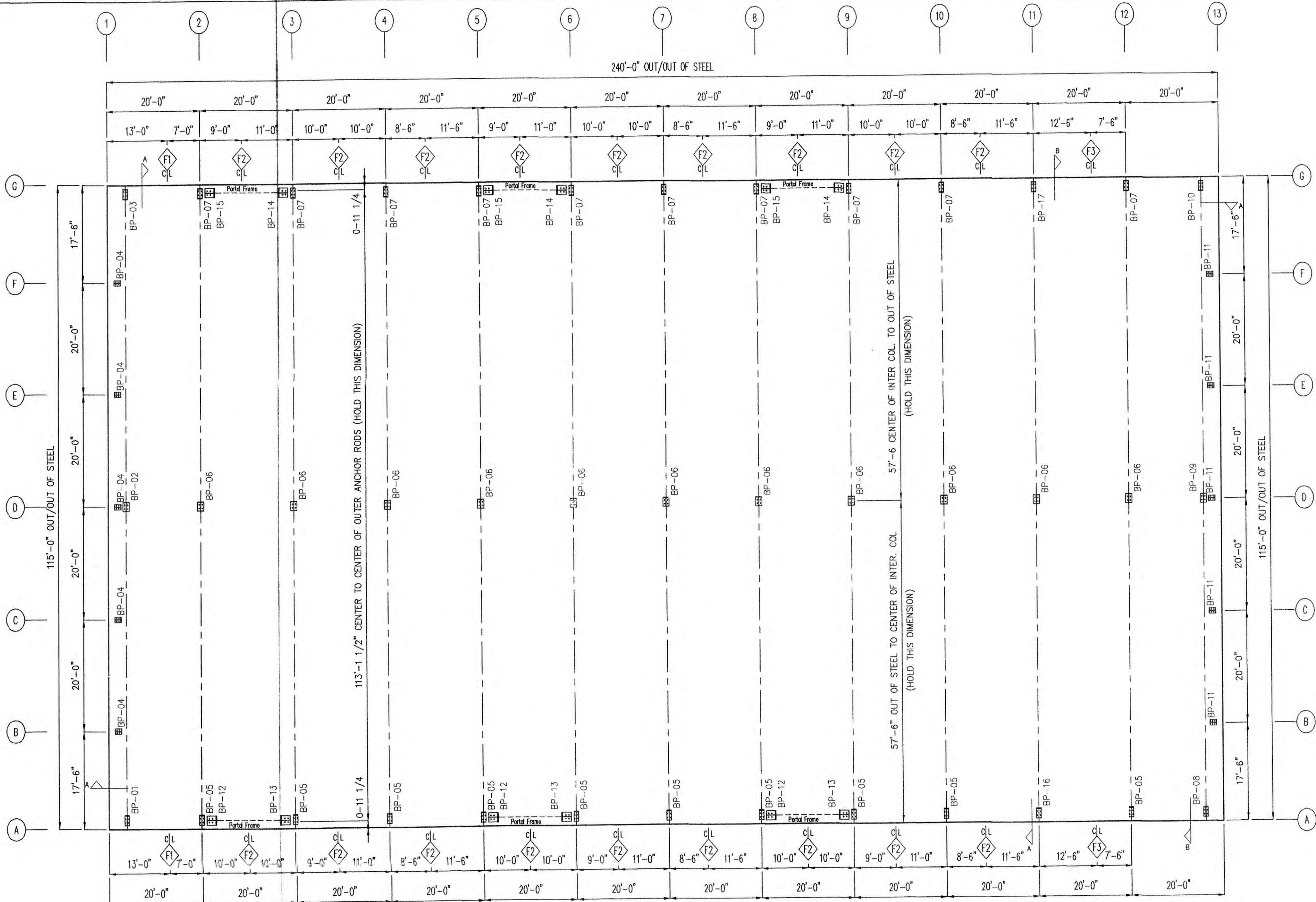
Design Code 2006 IBC 2nd Code 2002 ASCE-7

Roof Dead Load	Superimposed	2.02 psf
Collateral		5.00 psf
Roof Live Load		25.00 psf
Roof Live Loads reduced in accordance w/ IBC 1607.11.2.1		- YES X NO
Snow Load		
Ground Snow Load (Pg)		25.00 psf
Snow Load Importance Factor (I)		1.00
Flat-Roof Snow Load (Pf)		25.0 psf
Snow Exposure Factor (Ce)		1.00
Thermal Factor (Ct)		1.00
Wind Load		
Basic Wind Speed		95.00 mph
Wind Importance Factor (I)		1.00
Wind Exposure Category		C
Internal Pressure Coefficient (GCp)		+/- 0.18
Component Wind Load < 10'F1, Zone 4 +		19.70 psf pressure, -21.34 psf suction
Component Wind Load < 10'F1, Zone 5 +		19.70 psf pressure, -26.26 psf suction
+ Zones per ASCE 7-98, Figure 6-5A		
Seismic Load		
Seismic Use Group		I
Seismic Importance Factor (I _p)		1.00
S _a		1.050g
S ₁		0.350g
Site Class		D
Design Category		D
Analysis Procedure: Equivalent Lateral Force		
Basic Force Resisting System *	Transverse**	Longitudinal**
Response Modification Coefficient (R)	1	1
Deflection Amplification Factor (Cd)	3.50	3.50
System Over-strength Factor (Q _p)	3.00	3.00
Entire BLDG Design Base Shear (V)	3.00	3.00
	0.2326W	0.2326W

**-- The transverse direction is parallel to the rigid frames; the longitudinal direction is perpendicular to rigid frames.

Rainfall Intensity		
5-minute duration, 5-year recurrence (I ₅)	4	in/hour
5-minute duration, 25 year recurrence (I ₂₅)	6	in/hour
Floor Loads		
Superimposed Dead Load	-	psf
Collateral Dead Load	-	psf
Partition Load	-	psf
Total Dead Load	-	psf
Live Load	-	psf
Crane Loads		
Capacity & Type	5 TONS	
Max. Wheel Load	6,882 LBS.	
Wheelbase	6'-0"	
Bridge Weight	5,700 LBS.	
Hoist & Trolley Weight	900 LBS.	
Crane Class	UNDER HUNG, CLASS "C"	
Special Loads	=	
	=	
	=	

- * Basic Force Resisting System
1. Steel Ordinary Moment Frames
 2. Steel Ordinary Concentric Braced Frames
 3. Bearing Wall System w/ Sheet Steel Shearwalls (Diaphragm)



ANCHOR ROD SETTING PLAN (N.T.S)

These drawings and the metal building they represent are the product of Metallic Building Company or another division or affiliate of NQ Building Systems, LP. - 7301 Fairview, Houston, TX 77041. The engineer whose seal appears hereon is employed by Metallic Building Company or another division or affiliate of NQ Building Systems, LP. and is not the engineer of record for this project.

DRAWING STATUS		ISSUE	DATE	DESCRIPTION	BY	CHK'D
☐	For Approval:	0	8/31/07	FOR ERECTOR INSTALLATION	JAC	DGW
☐	For Construction Permit:					
☒	For Erector Installation:					

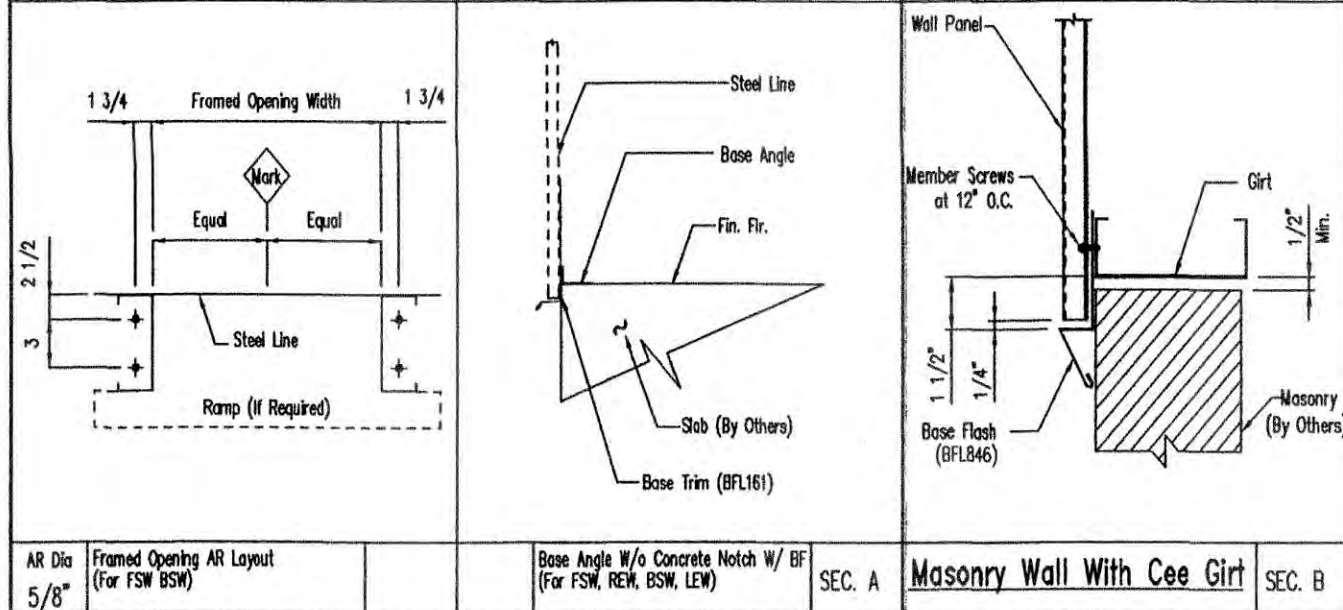
METALLIC		metallic building company	
7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338		ZIP 77041 (713) 466-7788 ZIP 77240	
Project: TRUCK SHOP/ OFFICE			
Customer: HEDGES CREEK MANAGEMENT # 1 LLC		Owner: HEDGES CREEK MANAGEMENT # 1 LLC	
Location: SCAPPOSE, OR			
CAD	Date	Scale	Phase
		N.T.S.	-
Building ID	Job Number	Sheet Number	Issue
A	0805-236857	F1 OF 3	0

The engineer whose seal appears hereon is not the engineer of record for the overall project.

9164

OREGON
JULY 28, 1971
S. HOEKSEMA

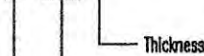
EXPIRES: 06/30/09



DRAWING STATUS

How to read part marks for:

Purlins, Girts, Eave Struts, Jamb, Headers



Thickness ID

Shape ID

Depth ID

DEPTH ID

ID

Depth

DESCRIPTION

ID

Description

SHAPE ID

ID

Description

THK ID

ID

Thk

08

0'-8"

2Z

2 1/8 x 2 3/8 Zee

3EL

3 x 5 Eave Strut - Low Side

6

16 GA

10

0'-10"

3Z

3 1/8 x 3 3/8 Zee

3EL

3 x 5 Eave Strut - High Side

5

15 GA

12

1'-0"

2C

2 1/2 Cee

3EL

5 x 5 Eave Strut - Low Side

4

14 GA

14

1'-2"

3C

3 1/2 Cee

5EH

5 x 5 Eave Strut - High Side

3

13 GA

3J

3 1/2 Cee Jamb

2

12 GA

3H

3 1/2 Cee Header/Sill

All letters and/or number which may appear after "Thickness ID" are for internal use only

How to read part marks for:

Cable and Rod Bracing



Diameter ID

Bracing ID

DEPTH ID

ID

Depth

DESCRIPTION

ID

Description

SHAPE ID

ID

Description

THK ID

ID

Thk

All letters and/or number which may appear after "Diameter ID" are for internal use only

Bracing ID		Dia ID	
ID	Type	ID	Dia
08	Cable	04	1/4"
BR	Rod	05	5/16"
		06	3/8"
		08	1/2"
		10	5/8"
		12	3/4"
		16	1"

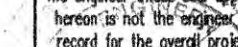
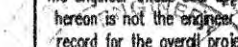
DRAWING STATUS

☐ For Approval.
 These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.

☐ For Construction Permit.

☒ For Erector Installation.

[illegible]

		metallic building company				The engineer whose name appears hereon is not the engineer of record for the overall project.	
7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338 ZIP 77041 (713) 466-7788 ZIP 77240		9164					
Project: TRUCK SHOP / OFFICE							
Customer: HEDGES CREEK MANAGEMENT # 1 LLC				Owner: HEDGES CREEK MANAGEMENT # 1 LLC			
Location: SCAPPOOSE, OR							
CAD	Date	Scale	Phase	Building ID	Job Number	Sheet Number	Issue
		N.T.S.	-	A	0805-236857	F2 OF 3	0
EXPIRATION DATE: 12/22/2017							

PORTAL FRAME AT LINES A&G				
NOTE: All reactions are shown in Kips. A negative value, indicates opposite direction. 				
Condition	Grid	Ver	Hor	Mom
DL	2,5,8,9,5,3	1.3	0.0	0.0
DL	3,6,9,5,2	1.3	0.0	0.0
0.6*DL + WL	2,5,8,9,5,3	-3.2	-1.7	0.0
0.6*DL + WL	3,6,9,5,2	4.8	-1.7	0.0
0.6*DL + WLR	2,5,8,9,5,3	4.8	1.7	0.0
0.6*DL + WLR	3,6,9,5,2	-3.2	1.7	0.0
0.749*DL + 1.0*EQ	2,5,8,9,5,3	-12.7	-5.8	0.0
0.749*DL + 1.0*EQ	3,6,9,5,2	14.6	-5.8	0.0
0.749*DL + 1.0*EQ	2,5,8,9,5,3	14.6	5.8	0.0
0.749*DL + 1.0*EQ	3,6,9,5,2	-12.7	5.8	0.0
1.35*DL + 1.35*WL + 1.0*EQ	2,5,8,9,5,3	-11.9	-5.8	0.0
1.35*DL + 1.35*WL + 1.0*EQ	3,6,9,5,2	15.4	-5.8	0.0
1.35*DL + 1.35*WL + 1.0*EQ	2,5,8,9,5,3	15.4	5.8	0.0
1.35*DL + 1.35*WL + 1.0*EQ	3,6,9,5,2	-11.9	5.8	0.0

Main Frame Reactions at 1,2				
NOTE: All reactions are shown in Kips. Moments are in Kip-Ft. A negative value, indicates opposite direction.				
Condition	Grid	Ver	Hor	
DL-Roof Dead Load	A	2.2	0.5	
DL-Roof Dead Load	D	5.0	0.0	
DL-Roof Dead Load	G	2.2	-0.5	
COLL-Roof Collateral Load	A	2.5	0.7	
COLL-Roof Collateral Load	D	5.9	0.0	
COLL-Roof Collateral Load	G	2.5	-0.7	
PL11-Pattern Live Load *LLx	A	13.3	1.8	
PL11-Pattern Live Load *LLx	D	14.7	0.0	
PL11-Pattern Live Load *LLx	G	-0.7	-1.8	
PL11-Pattern Live Load *LLx	A	-0.7	1.8	
PL11-Pattern Live Load *LLx	D	14.7	0.0	
PL11-Pattern Live Load *LLx	G	13.3	-1.8	
SNOW-Roof Snow Load	A	12.6	3.5	
SNOW-Roof Snow Load	D	29.4	0.0	
SNOW-Roof Snow Load	G	12.6	-3.5	
LL-Roof Live Load	A	12.6	3.5	
LL-Roof Live Load	D	29.4	0.0	
LL-Roof Live Load	G	12.6	-3.5	
EQ-Lateral Seismic Load parallel to plane of frame	A	-1.0	-2.4	
EQ-Lateral Seismic Load parallel to plane of frame	D	0.0	0.0	
EQ-Lateral Seismic Load parallel to plane of frame	G	1.0	-2.4	
W1-Lateral Primary Wind Load	A	-8.7	-3.9	
W1-Lateral Primary Wind Load	D	-14.8	0.0	
W1-Lateral Primary Wind Load	G	-4.6	-1.1	
W2-Lateral Primary Wind Load	A	-5.1	-4.1	
W2-Lateral Primary Wind Load	D	-7.7	-0.8	
W2-Lateral Primary Wind Load	G	-1.0	-0.8	
W3-Lateral Primary Wind Load	A	-10.5	-0.3	
W3-Lateral Primary Wind Load	D	-17.8	0.0	
W3-Lateral Primary Wind Load	G	-8.5	0.5	
W4-Lateral Primary Wind Load	A	-8.5	-0.5	
W4-Lateral Primary Wind Load	D	-17.8	0.0	
W4-Lateral Primary Wind Load	G	-10.5	0.3	
W5-Lateral Primary Wind Load	A	-6.4	0.6	
W5-Lateral Primary Wind Load	D	-10.9	0.0	
W5-Lateral Primary Wind Load	G	-5.5	-0.5	
W6-Lateral Primary Wind Load	A	-5.5	0.5	
W6-Lateral Primary Wind Load	D	-10.9	0.0	
W6-Lateral Primary Wind Load	G	-6.4	-0.6	
W7-Lateral Primary Wind Load	A	-4.6	1.1	
W7-Lateral Primary Wind Load	D	-14.8	0.0	
W7-Lateral Primary Wind Load	G	-4.6	-1.1	
W8-Lateral Primary Wind Load	A	-8.7	3.9	
W8-Lateral Primary Wind Load	D	-1.0	0.8	
W8-Lateral Primary Wind Load	G	-8.7	3.9	
W9-Lateral Primary Wind Load	A	-7.7	0.0	
W9-Lateral Primary Wind Load	D	-5.1	4.1	
W9-Lateral Primary Wind Load	G	-7.7	0.0	
RS-Unbalanced Right Roof Snow Load	A	2.0	2.5	
RS-Unbalanced Right Roof Snow Load	D	20.8	0.0	
RS-Unbalanced Right Roof Snow Load	G	15.9	-2.5	
LS-Unbalanced Left Roof Snow Load	A	15.9	2.5	
LS-Unbalanced Left Roof Snow Load	D	20.8	0.0	
LS-Unbalanced Left Roof Snow Load	G	2.0	-2.5	

Main Frame Reactions at 3-10				
NOTE: All reactions are shown in Kips. Moments are in Kip-Ft. A negative value, indicates opposite direction.				
Condition	Grid	Ver	Hor	
DL-Roof Dead Load	A	2.7	0.7	
DL-Roof Dead Load	D	5.4	0.0	
DL-Roof Dead Load	G	2.7	-0.7	
COLL-Roof Collateral Load	A	2.8	0.9	
COLL-Roof Collateral Load	D	6.0	0.0	
COLL-Roof Collateral Load	G	2.8	-0.9	
ADW-Auxiliary Dead Weight	A	0.7	0.3	
ADW-Auxiliary Dead Weight	D	2.3	0.0	
ADW-Auxiliary Dead Weight	G	0.7	-0.3	
PL11-Pattern Live Load *LLx	A	14.3	2.2	
PL11-Pattern Live Load *LLx	D	14.9	0.0	
PL11-Pattern Live Load *LLx	G	-0.4	-2.2	
PL11-Pattern Live Load *LLx	A	-0.4	2.2	
PL11-Pattern Live Load *LLx	D	14.9	0.0	
PL11-Pattern Live Load *LLx	G	14.3	-2.2	
SNOW-Roof Snow Load	A	13.8	4.5	
SNOW-Roof Snow Load	D	29.8	0.0	
SNOW-Roof Snow Load	G	13.8	-4.5	
AI11L-Auxiliary Crane #1 Dead Loading #1 Max. @ Left	A	2.8	0.5	
AI11L-Auxiliary Crane #1 Dead Loading #1 Max. @ Left	D	3.8	0.0	
AI11L-Auxiliary Crane #1 Dead Loading #1 Max. @ Left	G	0.0	-0.5	
AI11R-Auxiliary Crane #1 Dead Loading #1 Max. @ Right	A	2.2	0.4	
AI11R-Auxiliary Crane #1 Dead Loading #1 Max. @ Right	D	4.4	0.0	
AI11R-Auxiliary Crane #1 Dead Loading #1 Max. @ Right	G	0.0	0.4	
AI11L-Auxiliary Crane #2 Dead Loading #1 Max. @ Left	A	0.0	0.4	
AI11L-Auxiliary Crane #2 Dead Loading #1 Max. @ Left	D	4.4	0.0	
AI11L-Auxiliary Crane #2 Dead Loading #1 Max. @ Left	G	2.2	-0.4	
AI11R-Auxiliary Crane #2 Dead Loading #1 Max. @ Right	A	0.0	0.5	
AI11R-Auxiliary Crane #2 Dead Loading #1 Max. @ Right	D	3.8	0.0	
AI11R-Auxiliary Crane #2 Dead Loading #1 Max. @ Right	G	2.8	-0.5	
LL-Roof Live Load	A	13.8	4.5	
LL-Roof Live Load	D	29.8	0.0	
LL-Roof Live Load	G	13.8	-4.5	
AI11L-Auxiliary Crane #1 Vert Loading #1 Max. @ Left	A	9.0	1.7	
AI11L-Auxiliary Crane #1 Vert Loading #1 Max. @ Left	D	5.9	0.0	
AI11L-Auxiliary Crane #1 Vert Loading #1 Max. @ Left	G	0.2	-1.7	
AI11R-Auxiliary Crane #1 Vert Loading #1 Max. @ Right	A	-0.4	-0.9	
AI11R-Auxiliary Crane #1 Vert Loading #1 Max. @ Right	D	-0.1	0.0	
AI11R-Auxiliary Crane #1 Vert Loading #1 Max. @ Right	G	0.4	-0.9	
AI11L-Auxiliary Crane #1 Horz Loading #1 to the Left	A	0.4	0.9	
AI11L-Auxiliary Crane #1 Horz Loading #1 to the Left	D	0.1	0.0	
AI11L-Auxiliary Crane #1 Horz Loading #1 to the Left	G	-0.4	0.9	
AI11R-Auxiliary Crane #1 Horz Loading #1 to the Right	A	2.4	0.4	
AI11R-Auxiliary Crane #1 Horz Loading #1 to the Right	D	12.8	0.0	
AI11R-Auxiliary Crane #1 Horz Loading #1 to the Right	G	-0.1	-0.4	
A2V1L-Auxiliary Crane #2 Vert Loading #1 Max. @ Left	A	-0.1	0.4	
A2V1L-Auxiliary Crane #2 Vert Loading #1 Max. @ Left	D	12.8	0.0	
A2V1L-Auxiliary Crane #2 Vert Loading #1 Max. @ Left	G	2.4	-0.4	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	A	0.4	-0.9	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	D	0.1	0.0	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	G	-0.4	0.9	
A2H1L-Auxiliary Crane #2 Horz Loading #1 to the Left	A	0.4	-0.9	
A2H1L-Auxiliary Crane #2 Horz Loading #1 to the Left	D	0.1	0.0	
A2H1L-Auxiliary Crane #2 Horz Loading #1 to the Left	G	0.4	0.9	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	A	0.2	1.7	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	D	5.9	0.0	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	G	9.0	-1.7	
EQ-Lateral Seismic Load parallel to plane of frame	A	-1.9	-4.5	
EQ-Lateral Seismic Load parallel to plane of frame	D	0.0	0.0	
EQ-Lateral Seismic Load parallel to plane of frame	G	1.9	-4.5	

Main Frame Reactions at 11				
NOTE: All reactions are shown in Kips. Moments are in Kip-Ft. A negative value, indicates opposite direction.				
Condition	Grid	Ver	Hor	
DL-Roof Dead Load	A	3.3	0.6	
DL-Roof Dead Load	D	5.3	0.0	
DL-Roof Dead Load	G	3.3	-0.6	
COLL-Roof Collateral Load	A	5.9	0.9	
COLL-Roof Collateral Load	D	6.0	0.0	
COLL-Roof Collateral Load	G	5.9	-0.9	
ADW-Auxiliary Dead Weight	A	0.7	0.3	
ADW-Auxiliary Dead Weight	D	2.3	0.0	
ADW-Auxiliary Dead Weight	G	0.7	-0.3	
PL11-Pattern Live Load Left Leanto/Canopy *LLx	A	3.1	-0.4	
PL11-Pattern Live Load Left Leanto/Canopy *LLx	D	0.1	0.0	
PL11-Pattern Live Load Left Leanto/Canopy *LLx	G	-0.2	0.4	
PL11-Pattern Live Load *LLx	A	14.3	2.3	
PL11-Pattern Live Load *LLx	D	14.8	0.0	
PL11-Pattern Live Load *LLx	G	-0.4	-2.3	
PL11-Pattern Live Load *LLx	A	-0.4	2.3	
PL11-Pattern Live Load *LLx	D	14.8	0.0	
PL11-Pattern Live Load *LLx	G	14.3	-2.3	
PL11-Pattern Live Load Right Leanto/Canopy *LLx	A	-0.2	-0.4	
PL11-Pattern Live Load Right Leanto/Canopy *LLx	D	0.1	0.0	
PL11-Pattern Live Load Right Leanto/Canopy *LLx	G	3.1	0.4	
SNOW-Roof Snow Load	A	16.9	4.1	
SNOW-Roof Snow Load	D	29.7	0.0	
SNOW-Roof Snow Load	G	16.9	-4.1	
AI11L-Auxiliary Crane #1 Dead Loading #1 Max. @ Left	A	2.8	0.6	
AI11L-Auxiliary Crane #1 Dead Loading #1 Max. @ Left	D	3.8	0.0	
AI11L-Auxiliary Crane #1 Dead Loading #1 Max. @ Left	G	0.0	-0.6	
AI11R-Auxiliary Crane #1 Dead Loading #1 Max. @ Right	A	2.2	0.4	
AI11R-Auxiliary Crane #1 Dead Loading #1 Max. @ Right	D	4.4	0.0	
AI11R-Auxiliary Crane #1 Dead Loading #1 Max. @ Right	G	0.0	0.4	
AI11L-Auxiliary Crane #2 Dead Loading #1 Max. @ Left	A	0.0	0.4	
AI11L-Auxiliary Crane #2 Dead Loading #1 Max. @ Left	D	4.4	0.0	
AI11L-Auxiliary Crane #2 Dead Loading #1 Max. @ Left	G	2.2	-0.4	
AI11R-Auxiliary Crane #2 Dead Loading #1 Max. @ Right	A	0.0	0.5	
AI11R-Auxiliary Crane #2 Dead Loading #1 Max. @ Right	D	3.8	0.0	
AI11R-Auxiliary Crane #2 Dead Loading #1 Max. @ Right	G	2.8	-0.5	
LL-Roof Live Load	A	13.8	4.5	
LL-Roof Live Load	D	29.8	0.0	
LL-Roof Live Load	G	13.8	-4.5	
AI11L-Auxiliary Crane #1 Vert Loading #1 Max. @ Left	A	9.0	1.7	
AI11L-Auxiliary Crane #1 Vert Loading #1 Max. @ Left	D	5.9	0.0	
AI11L-Auxiliary Crane #1 Vert Loading #1 Max. @ Left	G	0.2	-1.7	
AI11R-Auxiliary Crane #1 Vert Loading #1 Max. @ Right	A	-0.4	-0.9	
AI11R-Auxiliary Crane #1 Vert Loading #1 Max. @ Right	D	-0.1	0.0	
AI11R-Auxiliary Crane #1 Vert Loading #1 Max. @ Right	G	0.4	-0.9	
AI11L-Auxiliary Crane #1 Horz Loading #1 to the Left	A	0.4	0.9	
AI11L-Auxiliary Crane #1 Horz Loading #1 to the Left	D	0.1	0.0	
AI11L-Auxiliary Crane #1 Horz Loading #1 to the Left	G	-0.4	0.9	
AI11R-Auxiliary Crane #1 Horz Loading #1 to the Right	A	2.4	0.4	
AI11R-Auxiliary Crane #1 Horz Loading #1 to the Right	D	12.8	0.0	
AI11R-Auxiliary Crane #1 Horz Loading #1 to the Right	G	-0.1	-0.4	
A2V1L-Auxiliary Crane #2 Vert Loading #1 Max. @ Left	A	-0.1	0.4	
A2V1L-Auxiliary Crane #2 Vert Loading #1 Max. @ Left	D	12.8	0.0	
A2V1L-Auxiliary Crane #2 Vert Loading #1 Max. @ Left	G	2.4	-0.4	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	A	0.4	-0.9	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	D	0.1	0.0	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	G	-0.4	0.9	
A2H1L-Auxiliary Crane #2 Horz Loading #1 to the Left	A	0.4	-0.9	
A2H1L-Auxiliary Crane #2 Horz Loading #1 to the Left	D	0.1	0.0	
A2H1L-Auxiliary Crane #2 Horz Loading #1 to the Left	G	0.4	0.9	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	A	0.2	1.7	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	D	5.9	0.0	
A2V1R-Auxiliary Crane #2 Vert Loading #1 Max. @ Right	G	9.0	-1.7	
EQ-Lateral Seismic Load parallel to plane of frame	A	-1.9	-4.5	
EQ-Lateral Seismic Load parallel to plane of frame	D	0.0	0.0	
EQ-Lateral Seismic Load parallel to plane of frame	G	1.9	-4.5	

Main Frame Reactions at 12,13

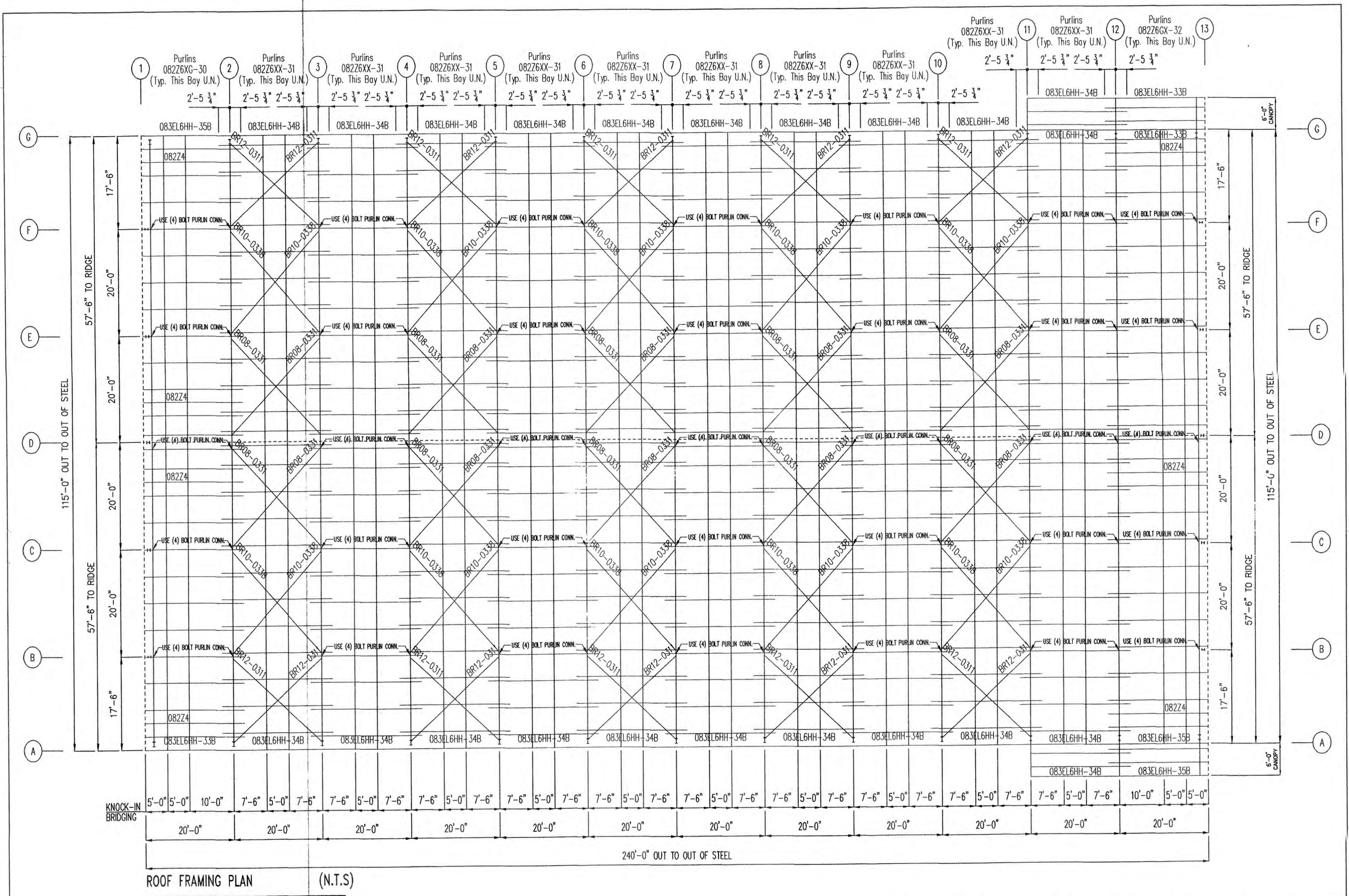
Hor +

Ver +

Support

NOTE: All reactions are shown in Kips. Moments are in Kip-Ft
A negative value, indicates opposite direction

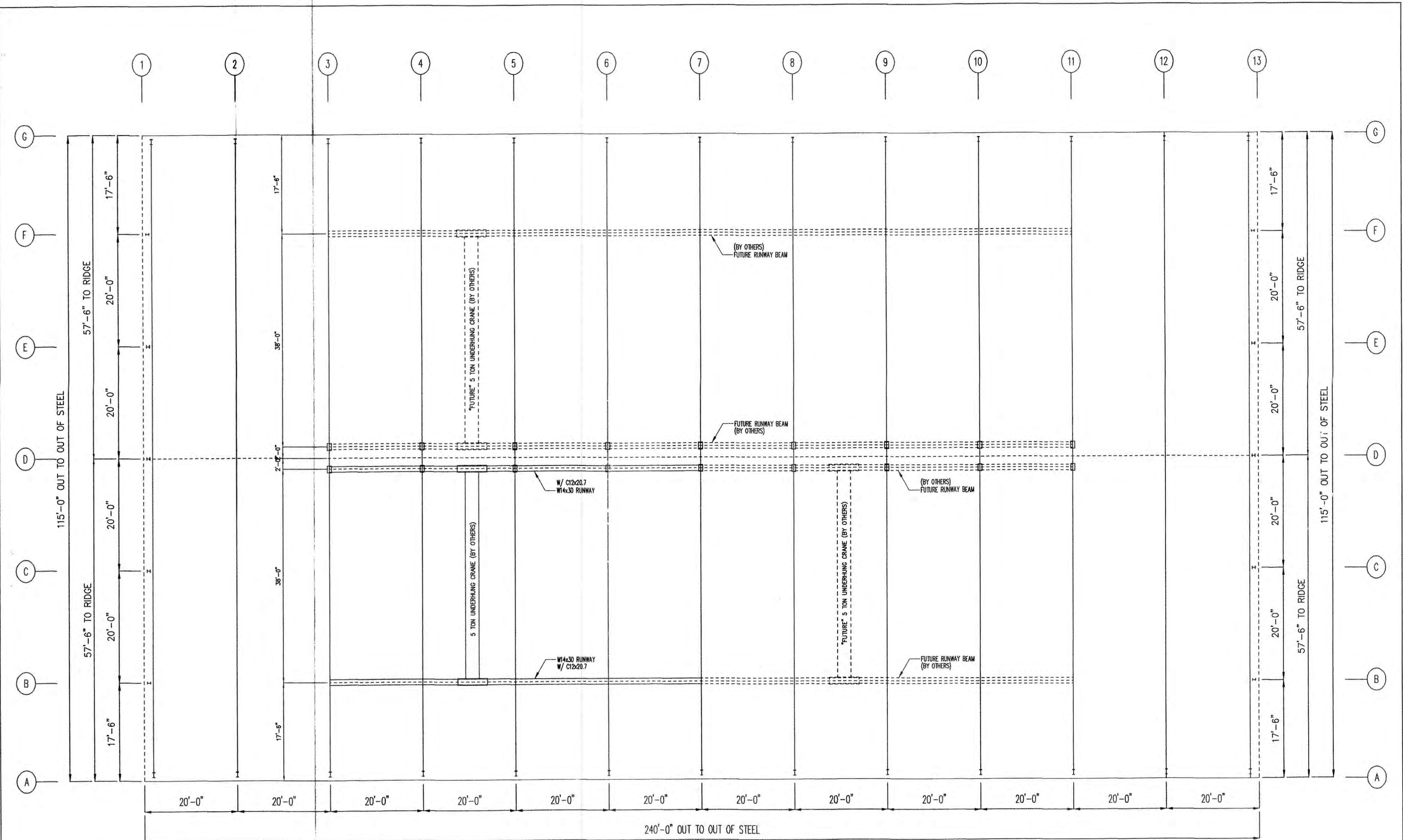
Condition	Grid	Ver	Hor
DL-Roof Dead Load	A	2.9	0.4
DL-Roof Dead Load	D	5.0	0.0
DL-Roof Dead Load	G	2.9	-0.4
COLL-Roof Collateral Load	A	2.5	0.7
COLL-Roof Collateral Load	D	5.8	0.0
COLL-Roof Collateral Load	G	2.5	-0.7
PL11-Pattern Live Load Left Leanto/Canopy *LLx	A	3.7	-0.5
PL11-Pattern Live Load Left Leanto/Canopy *LLx	D	0.1	0.0
PL11-Pattern Live Load Left Leanto/Canopy *LLx	G	-0.2	0.5
PL11-Pattern Live Load *LLx	A	13.3	1.8
PL11-Pattern Live Load *LLx	D	14.6	0.0
PL11-Pattern Live Load *LLx	G	-0.6	-1.8
PL11-Pattern Live Load *LLx	A	-0.6	1.8
PL11-Pattern Live Load *LLx	D	14.6	0.0
PL11-Pattern Live Load *LLx	G	13.3	-1.8
PL11-Pattern Live Load Right Leanto/Canopy *LLx	A	-0.2	-0.5
PL11-Pattern Live Load Right Leanto/Canopy *LLx	D	0.1	0.0
PL11-Pattern Live Load Right Leanto/Canopy *LLx	G	3.7	0.5
SNOW-Roof Snow Load	A	16.2	2.9
SNOW-Roof Snow Load	D	29.4	0.0
SNOW-Roof Snow Load	G	16.2	-2.9
LL-Roof Live Load	A	16.2	2.9
LL-Roof Live Load	D	29.4	0.0
LL-Roof Live Load	G	16.2	-2.9
EQ-Lateral Seismic Load parallel to plane of frame	A	-1.3	-3.1
EQ-Lateral Seismic Load parallel to plane of frame	D	0.0	0.0
EQ-Lateral Seismic Load parallel to plane of frame	G	1.3	-3.1
W1-Lateral Primary Wind Load	A	-12.6	-0.5
W1-Lateral Primary Wind Load	D	-14.8	0.0
W1-Lateral Primary Wind Load	G	-5.4	-1.4
W2-Lateral Primary Wind Load	A	-9.0	-3.4
W2-Lateral Primary Wind Load	D	-7.7	-0.8
W2-Lateral Primary Wind Load	G	-1.9	-1.9
W3-Lateral Primary Wind Load	A	-13.3	0.2
W3-Lateral Primary Wind Load	D	-17.7	0.0
W3-Lateral Primary Wind Load	G	-10.3	0.1
W4-Lateral Primary Wind Load	A	-9.0	-3.4
W4-Lateral Primary Wind Load	D	-7.7	-0.8
W4-Lateral Primary Wind Load	G	-1.9	-1.9
W5-Lateral Primary Wind Load	A	-13.3	0.2
W5-Lateral Primary Wind Load	D	-17.7	0.0
W5-Lateral Primary Wind Load	G	-10.3	0.1
W6-Lateral Primary Wind Load	A	-9.0	-3.4
W6-Lateral Primary Wind Load	D	-7.7	-0.8
W6-Lateral Primary Wind Load	G	-1.9	-1.9
RS-Unbalanced Right Roof Snow Load	A	2.1	0.0
RS-Unbalanced Right Roof Snow Load	D	20.7	0.0
RS-Unbalanced Right Roof Snow Load	G	16.0	0.0
LS-Unbalanced Left Roof Snow Load	A	16.0	0.0
LS-Unbalanced Left Roof Snow Load	D	20.7	0.0
LS-Unbalanced Left Roof Snow Load	G	2.1	0.0
DSNL-Drifting Snow Left	A	7.1	-0.1
DSNL-Drifting Snow Left	D	0.1	0.0
DSNL-Drifting Snow Left	G	-0.3	0.0



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How to read part marks for:				DRAWING STATUS				ISSUE				metallic building company			
Purlins, Girts, Eave Struts, Jambos, Headers				Cable and Rod Bracing				A				7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338 ZIP 77041 (713) 468-7788 ZIP 77240			
DEPTH ID	SHAPE ID	THK ID	DESCRIPTION	BRACING ID	DIA ID	DATE	DESCRIPTION	BY	CHKD	DATE	DESCRIPTION	BY	CHKD	DATE	DESCRIPTION
08 0'-8"	22 2 1/8 x 2 3/8 Zee	3EL	3 x 5 Eave Strut - Low Side	04 1/4"	04 1/4"	8/31/07	FOR CONSTRUCTION PERMIT	JAC	DGN						
10 0'-10"	32 3 1/8 x 3 3/8 Zee	3EH	3 x 5 Eave Strut - High Side	05 5/16"	05 5/16"										
12 1'-0"	20 2 1/2 Cee	5EL	5 x 5 Eave Strut - Low Side	06 3/8"	06 3/8"										
14 1'-2"	36 3 1/2 Cee	5EH	5 x 5 Eave Strut - High Side	08 1/2"	08 1/2"										
	3J 3 1/2 Cee Jamb			10 5/8"	10 5/8"										
	3H 3 1/2 Cee Header/Sill			12 3/4"	12 3/4"										
				16 1"	16 1"										

Project: TRUCK SHOP / OFFICE
Customer: HEDGES CREEK MANAGEMENT # 1 LLC
Location: SCAPPOSE, OR
Job Number: 0805-236857
Sheet Number: E1 OF 11
Issue: A
EXP: 06/30/08



CRANE LAYOUT (N.T.S.)
FUTER CRANES

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How to read part marks for:				SHAPE ID				THK ID				How to read part marks for:				DRAWING STATUS				ISSUE				DESCRIPTION				BY				CKD			
Purlins, Girts, Eave Struts, Jambos, Headers				ID Description				ID Description				Cable and Rod Bracing				ID Type ID Dia				A 8/31/07				FOR CONSTRUCTION PERMIT				JAC DGN							
				08 0'-8" 2Z 2 1/8 x 2 3/8 Zee 10 0'-10" 3Z 3 1/8 x 3 3/8 Zee 12 1'-0" 2C 2 1/2 Cee 14 1'-2" 3C 3 1/2 Cee 3H 3 1/2 Cee Header/Sill				3EL 3 x 5 Eave Strut - Low Side 3EH 3 x 5 Eave Strut - High Side 5EL 5 x 5 Eave Strut - Low Side 5EH 5 x 5 Eave Strut - High Side				6 16 GA 5 15 GA 4 14 GA 3 13 GA 2 12 GA				ID Type ID Dia CB Cable 04 1/4" BR Rod 05 5/16" 06 3/8" 08 1/2" 10 5/8" 12 3/4" 16 1"				☐ For Approval: These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete. ☒ For Construction Permit: ☐ For Erector Installation:															

metallic building company

7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338
 ZIP 77041 (713) 468-7788 ZIP 77240

Project: TRUCK SHOP/ OFFICE

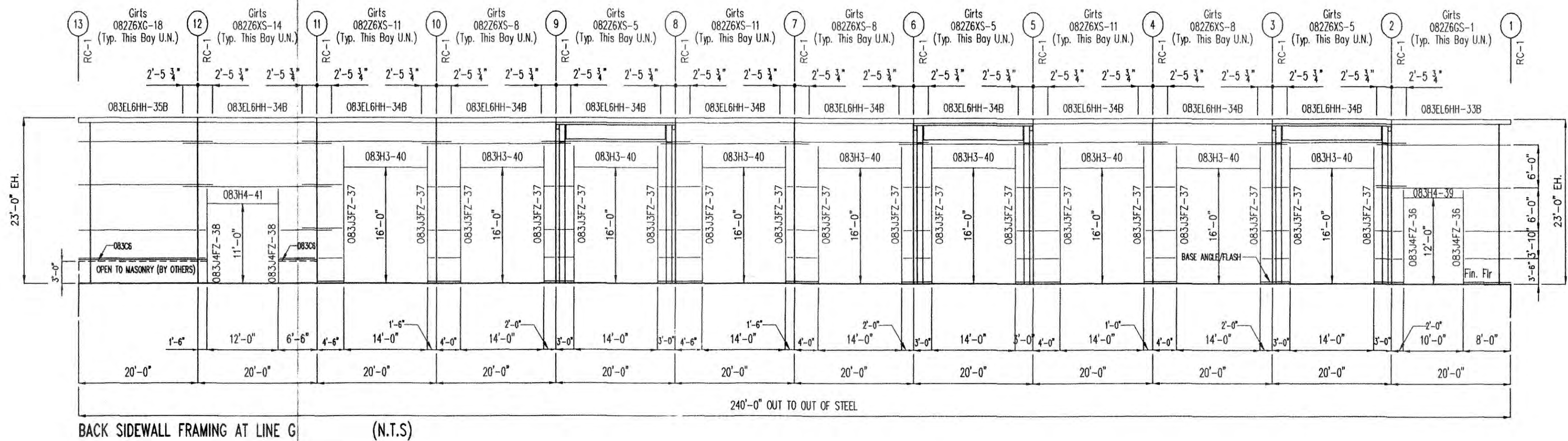
Customer: HEDGES CREEK MANAGEMENT # 1 LLC Owner: HEDGES CREEK MANAGEMENT # 1 LLC

Location: SCAPPOSE, OR

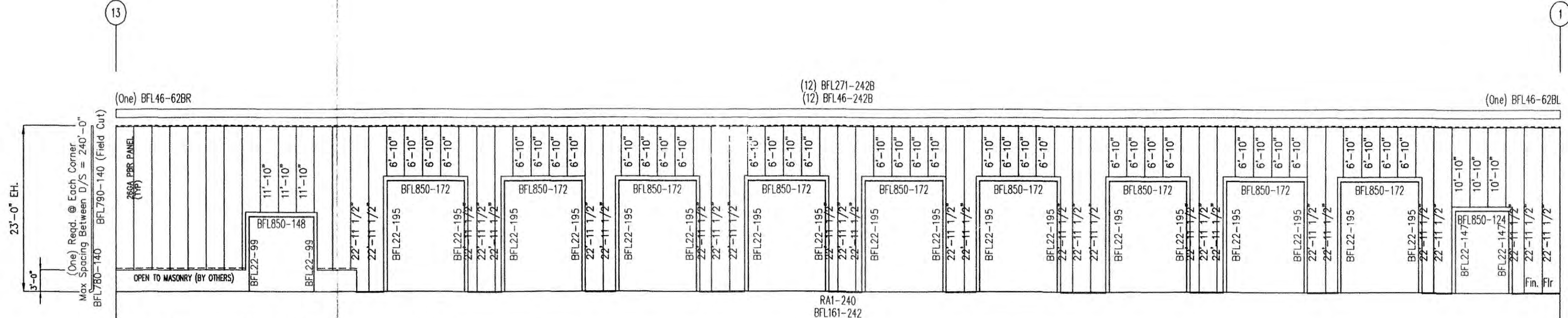
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EXPIRES 08/31/09

CAD	Date	Scale	Phase	Building ID	Job Number	Sheet Number	Issue
		N.T.S.	-	A	0805-236857	E2 OF 11	A



BACK SIDEWALL FRAMING AT LINE G (N.T.S.)



BACK SIDEWALL SHEETING AT LINE G (N.T.S.)

How to read part marks for:

Purlins, Girts, Eave Struts, Jamb, Headers	DEPTH ID	SHAPE ID	THK ID
08 0'-0"	22 2 1/8 x 2 3/8 Zee	3EL 3 x 5 Eave Strut - Low Side	6 16 GA
10 0'-10"	32 3 1/8 x 3 3/8 Zee	3EH 3 x 5 Eave Strut - High Side	5 15 GA
12 1'-0"	2C 2 1/2 Cee	5EL 5 x 5 Eave Strut - Low Side	4 14 GA
14 1'-2"	3C 3 1/2 Cee	5EH 5 x 5 Eave Strut - High Side	3 13 GA
	3J 3 1/2 Cee Jamb		2 12 GA
	3H 3 1/2 Cee Header/Sill		

All letters and/or number which may appear after "Thickness ID" are for internal use only

How to read part marks for:

Cable and Rod Bracing	BRACING ID	DIA ID
08 Cable	04 1/4"	
BR Rod	05 5/16"	
	06 3/8"	
	08 1/2"	
	10 5/8"	
	12 3/4"	
	16 1"	

All letters and/or number which may appear after "Diameter ID" are for internal use only

DRAWING STATUS

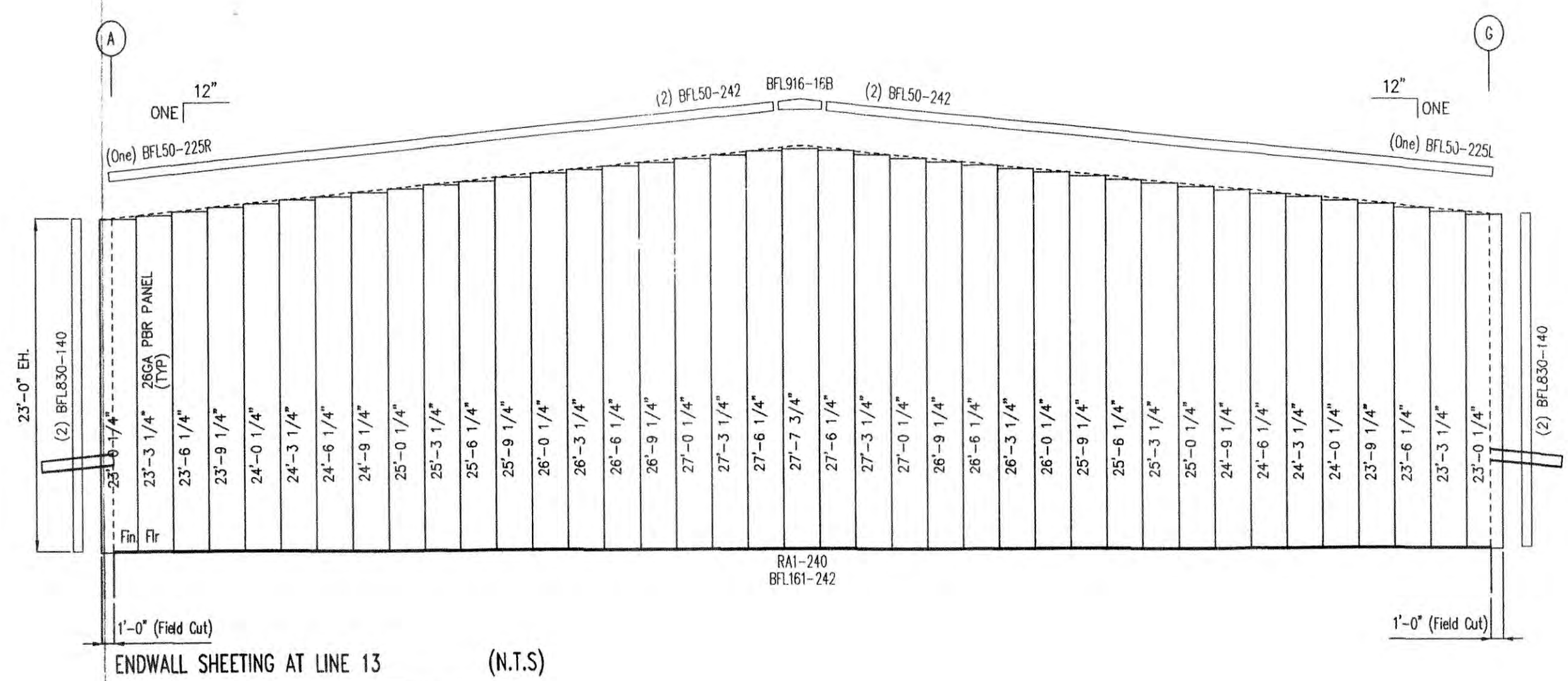
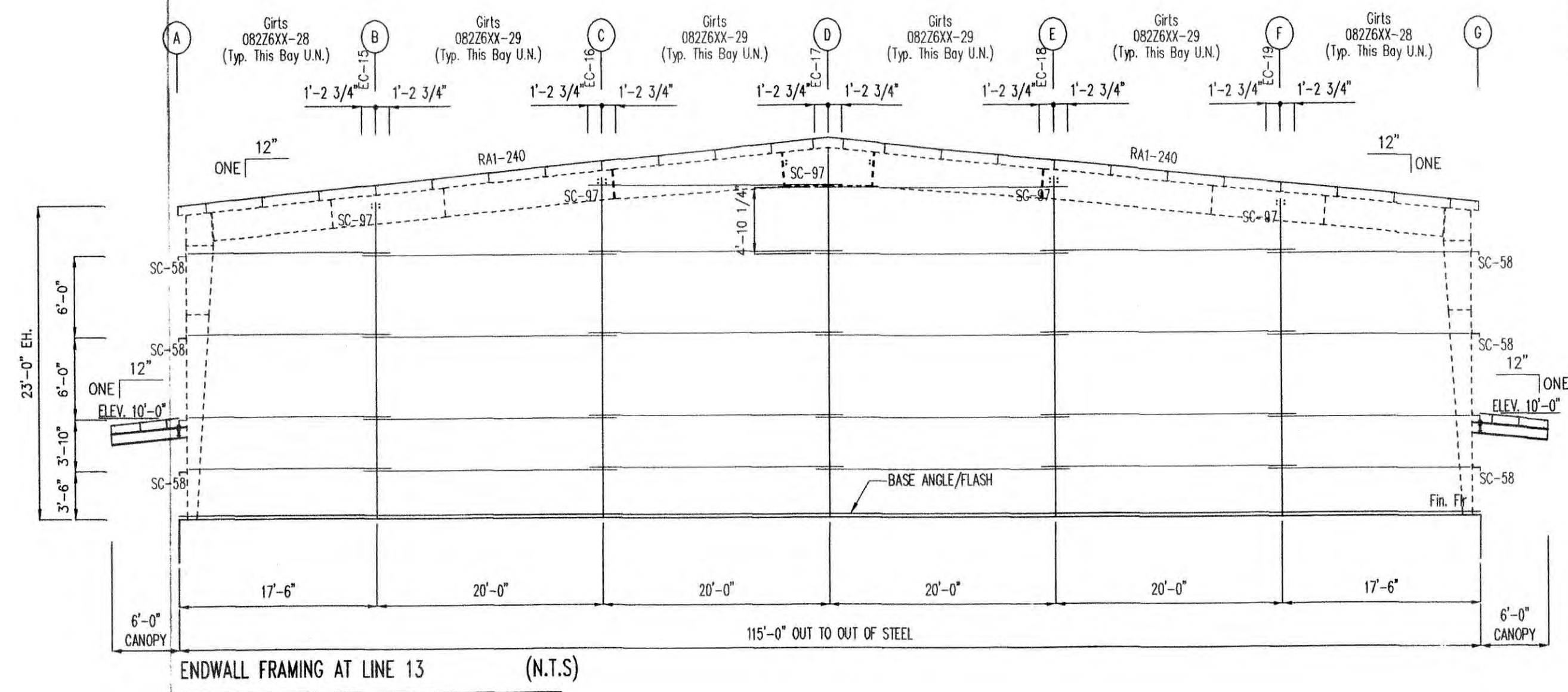
☐ For Approval:	These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.
☒ For Construction Permit:	
☐ For Erector Installation:	

ISSUE	DATE	DESCRIPTION	BY	CHKD
A	8/31/07	FOR CONSTRUCTION PERMIT	JAC	DGW
B	9/14/07	REVISED FOR CONSTRUCTION PERMIT	JAC	BB

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METALLIC		metallic building company	
7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338 ZIP 77041 (713) 466-7788		ZIP 77240	
Project: TRUCK SHOP / OFFICE			
Customer: HEDGES CREEK MANAGEMENT #1 LLC		Owner: HEDGES CREEK MANAGEMENT #1 LLC	
Location: SCAPPOOSE, OR			
CAD	Date	Scale	Phase
		N.T.S.	-
Building ID	Job Number	Sheet Number	Issue
A	0805-236857	E4 OF 11	B

MEMBER DATA			
Column data is listed from Base to Eave - Rafter data is listed from Eave to Ridge (High Side)			
Mark	Outside Flange	Web	Inside Flange
Col (EC-15)	W 8 x 15		
Col (EC-16)	W 8 x 18		
Col (EC-17)	W 8 x 18		
Col (EC-18)	W 8 x 18		
Col (EC-19)	W 8 x 15		



How to read part marks for:
Purlins, Girts, Eave Struts, Jamb, Headers

Thickness ID
Shape ID
Depth ID

All letters and/or number which may appear after "Thickness ID" are for internal use only

DEPTH ID	SHAPE ID	THK ID
ID	ID	ID
08 0'-8"	22 2 1/8 x 2 3/8 Zee	3EL 3 x 5 Eave Strut - Low Side
10 0'-10"	32 3 1/8 x 3 3/8 Zee	3EH 3 x 5 Eave Strut - High Side
12 1'-0"	20 2 1/2 Cee	5EL 5 x 5 Eave Strut - Low Side
14 1'-2"	30 3 1/2 Cee	5EH 5 x 5 Eave Strut - High Side
	3J 3 1/2 Cee Jamb	
	3H 3 1/2 Cee Header/Sill	

How to read part marks for:
Cable and Rod Bracing

Diameter ID
Bracing ID

All letters and/or number which may appear after "Diameter ID" are for internal use only

BRACING ID	DIA ID
ID	ID
CB Cable	04 1/4"
BR Rod	05 5/16"
	06 3/8"
	08 1/2"
	10 5/8"
	12 3/4"
	16 1"

DRAWING STATUS	
☐ For Approval:	These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.
☒ For Construction Permit:	
☐ For Erector Installation:	

ISSUE	DATE	DESCRIPTION	BY	CHKD
A	8/31/07	FOR CONSTRUCTION PERMIT	JAC	DGW

METALLIC metallic building company

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ZIP 77041 (713) 466-7788 ZIP 77240

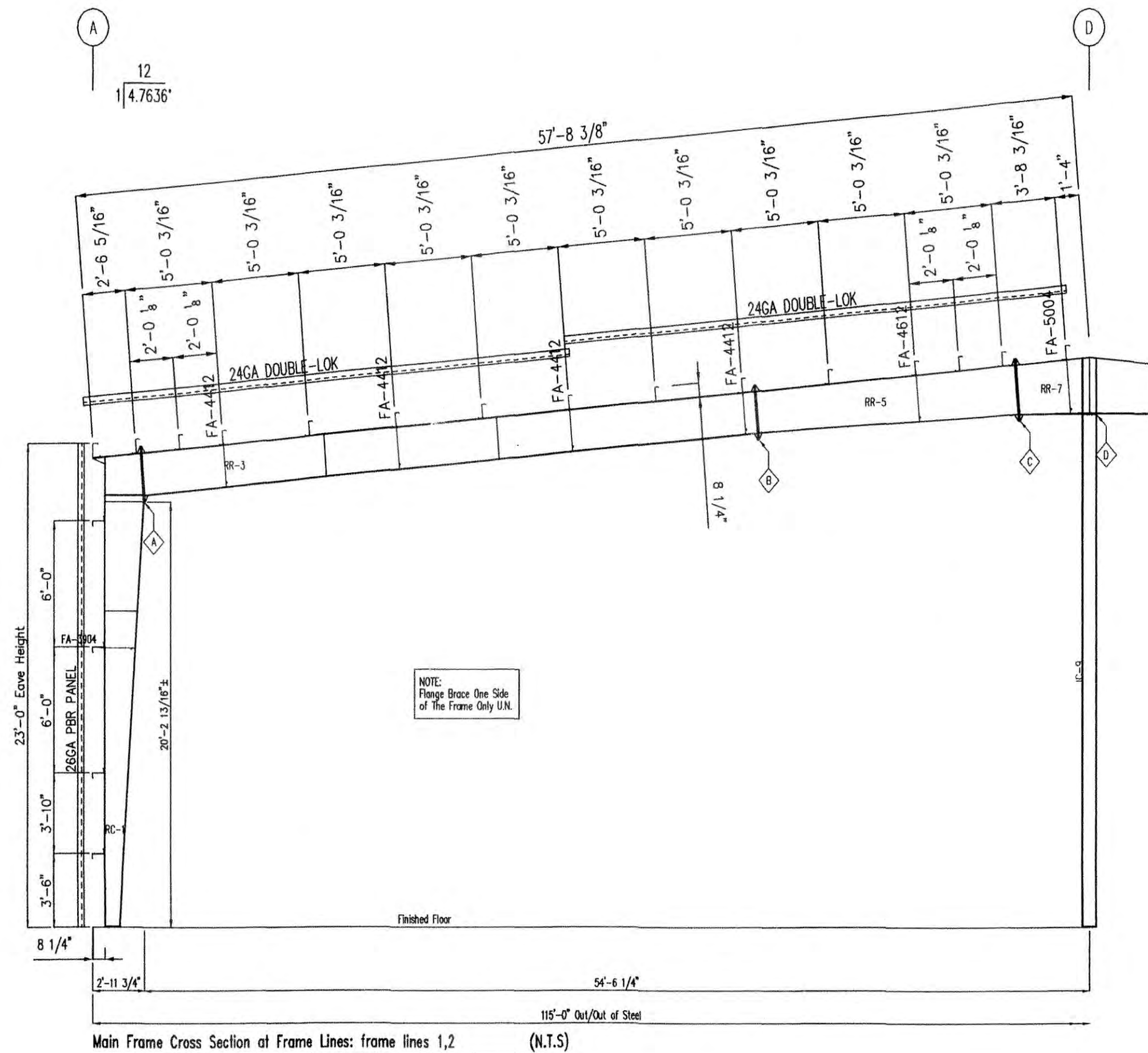
Project: TRUCK SHOP / OFFICE
Customer: HEDGES CREEK MANAGEMENT #1 LLC Owner: HEDGES CREEK MANAGEMENT #1 LLC
Location: SCAPPOOSE, OR

CAD Date Scale Phase Building ID Job Number Sheet Number Issue
N.T.S. - A 0805-236857 E6 OF 11 A

9164
JULY 22, 1971
ROBERT S. HOEKSTRA

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MEMBER DATA			
Column data is listed from Base to Eave - Rafter data is listed from Eave to Ridge (High Side)			
Mark	Outside Flange	Web	Inside Flange
Col (RC-1)	1/4 x 6 x 15-0 1/4 x 6	10 GA x 10 to 22 7/16 x 15-0 1/4 x 22 7/16 to 27	3/8 x 6 x 15-0 1/4 x 6
Col (RC-9)	W 10 x 33		
Raf (RR-3)	1/4 x 6 3/8 x 6 x 10-0 3/8 x 6 x 14-11 1/2	3/16 x 23 1/2 8 GA x 23 1/2 x 10-0 8 GA x 23 1/2 x 14-11 1/2	3/8 x 6 3/8 x 6 x 10-0 3/8 x 6 x 14-11 1/2
Raf (RR-5)	5/16 x 6 x 14-11 7/16	3/16 x 23 1/2 to 28 x 14-11 7/16	1/4 x 6 x 14-11 7/16
Raf (RR-7)	1/4 x 6 x 4-2 13/16 1/4 x 6 x 4-2 13/16	1/4 x 28 to 32 1/16 x 4-2 13/16 1/4 x 32 1/16 to 28 x 4-2 13/16	5/16 x 6 x 4-2 13/16 5/16 x 6 x 4-2 13/16



A	B	C	D

DRAWING STATUS			
☐ For Approval:	ISSUE	DATE	DESCRIPTION
☒ For Construction Permit:	A	8/31/07	FOR CONSTRUCTION PERMIT
☐ For Erector Installation:			

BY	CHKD	DATE	DESCRIPTION
JAC	DGW		

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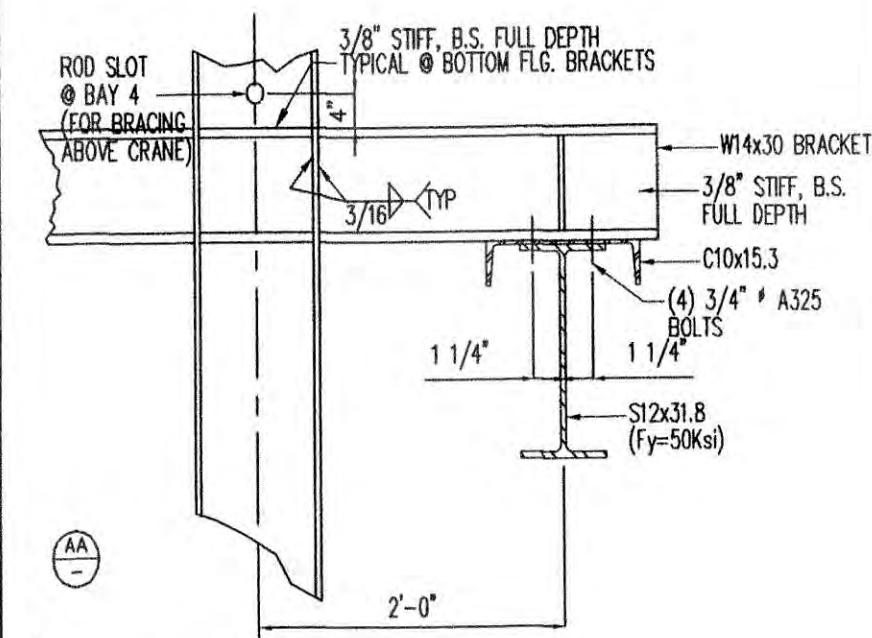
METALLIC metallic building company

7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Project: TRUCK SHOP/ OFFICE
Customer: HEDGES CREEK MANAGEMENT # 1 LLC Owner: HEDGES CREEK MANAGEMENT # 1 LLC
Location: SCAPPOSE, OR

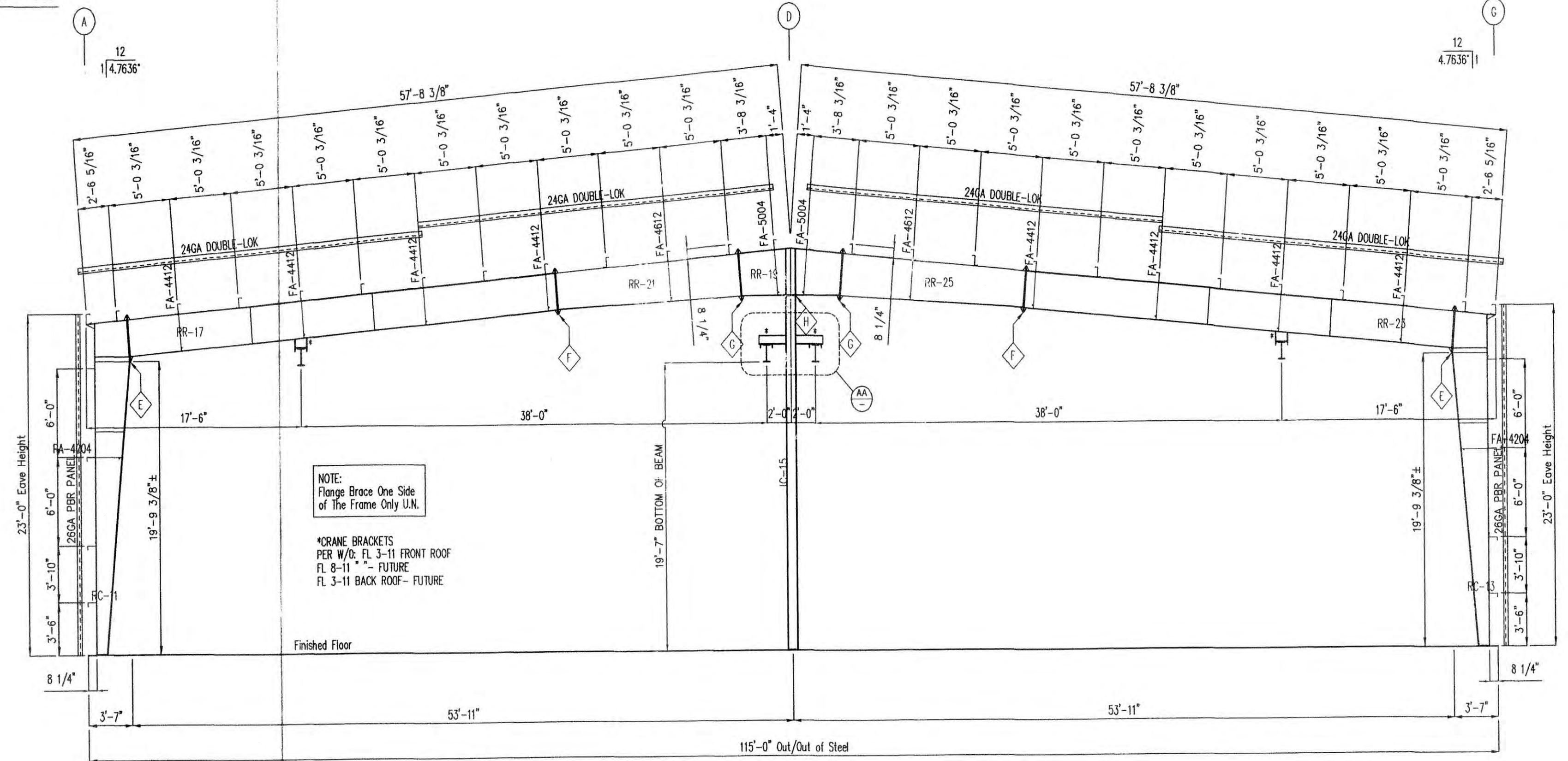
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N.T.S. - A 0805-236857 E7 OF 11 A

9164
JULY 22, 1971
GERT S. HOEKSTRA
ENGINEER

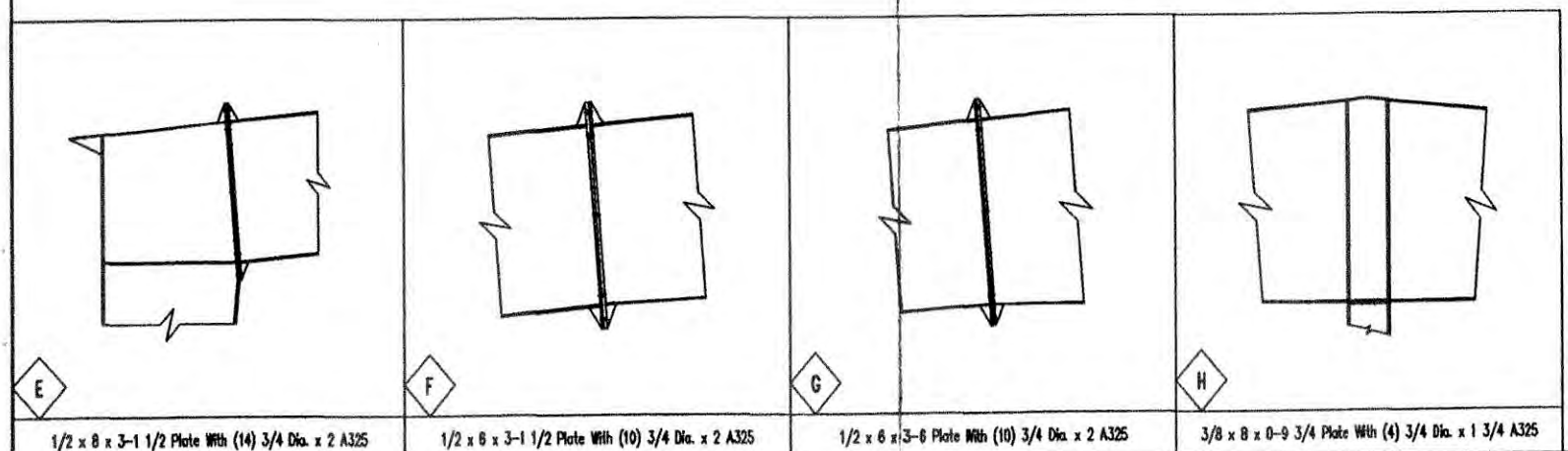


CRANE BRACKET @ INT. COL

MEMBER DATA			
Column data is listed from Base to Eave - Rafter data is listed from Eave to Ridge (High Side)			
Mark	Outside Flange	Web	Inside Flange
Col (RC-11)	1/4 x 8 x 15-0 1/4 x 8	10 GA x 10 to 27 15/16 x 15-0 1/4 x 27 15/16 to 34	1/2 x 8 x 15-0 1/2 x 8
Col (IC-15)	W 10 x 33		
Col (RC-13)	1/4 x 8 x 15-0 1/4 x 8	10 GA x 10 to 27 15/16 x 15-0 1/4 x 27 15/16 to 34	1/2 x 8 x 15-0 1/2 x 8
Raf (RR-17)	1/4 x 6 3/8 x 6 x 10-0 3/8 x 6 x 14-11 1/2	1/4 x 29 1/2 3/16 x 29 1/2 x 10-0 3/16 x 29 1/2 x 14-11 1/2	5/16 x 6 5/16 x 6 x 10-0 5/16 x 6 x 14-11 1/2
Raf (RR-21)	1/4 x 6 x 14-10 15/16 1/4 x 6 x 4-2 3/4 1/4 x 6 x 4-2 3/4	1/4 x 29 1/2 to 34 x 14-10 15/16 1/4 x 34 to 38 x 4-2 3/4 1/4 x 38 to 34 x 4-2 3/4	1/4 x 6 x 14-10 15/16 1/4 x 6 x 4-2 3/4 1/4 x 6 x 4-2 3/4
Raf (RR-23)	1/4 x 6 3/8 x 6 x 10-0 3/8 x 6 x 14-11 1/2	1/4 x 29 1/2 3/16 x 29 1/2 x 10-0 3/16 x 29 1/2 x 14-11 1/2	5/16 x 6 5/16 x 6 x 10-0 5/16 x 6 x 14-11 1/2
Raf (RR-25)	1/4 x 6 x 14-10 15/16	1/4 x 29 1/2 to 34 x 14-10 15/16	1/4 x 6 x 14-10 15/16



Main Frame Cross Section at Frame Lines: frame lines 3-10 (N.T.S)



DRAWING STATUS		ISSUE	DATE	DESCRIPTION	BY	CKD
☐ For Approval:	These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.	A	8/31/07	FOR CONSTRUCTION PERMIT	JAC	DGW
☒ For Construction Permit:						
☐ For Erector Installation:						

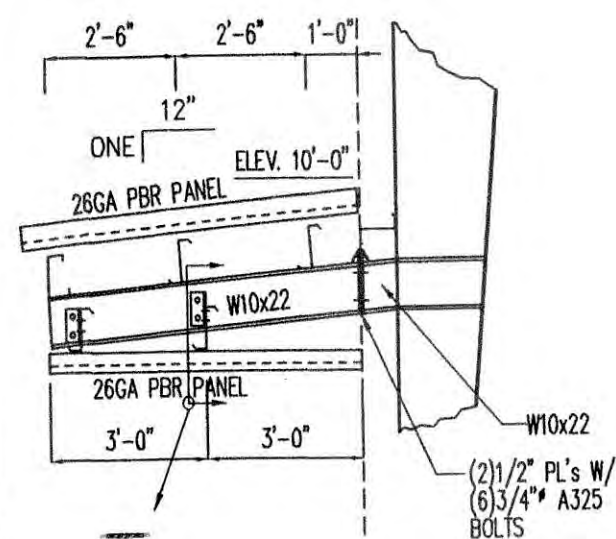
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METALLIC metallic building company
7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338
ZIP 77041 (713) 466-7788 ZIP 77240

Project: TRUCK SHOP / OFFICE
Customer: HEDGES CREEK MANAGEMENT # 1 LLC Owner: HEDGES CREEK MANAGEMENT # 1 LLC
Location: SCAPPOOSE, OR

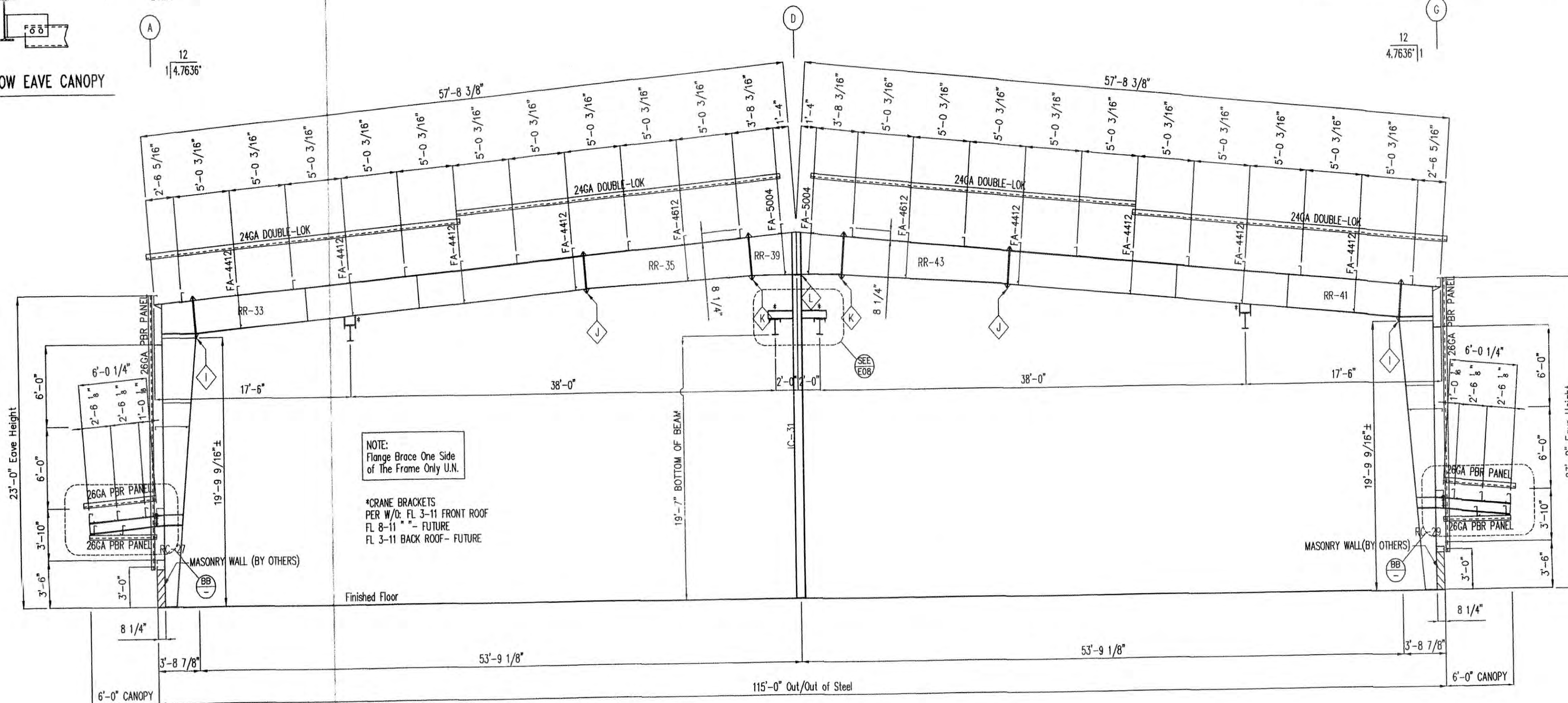
CAD	Date	Scale	Phase	Building ID	Job Number	Sheet Number	Issue
		N.T.S.	-	A	0805-236857	E8 OF 11	A

9484
JULY 22, 1984
S. HOBBS
REGISTERED PROFESSIONAL ENGINEER
STATE OF OREGON



BELOW EAVE CANOPY

MEMBER DATA			
Column data is listed from Base to Eave - Rafter data is listed from Eave to Ridge (High Side)			
Mark	Outside Flange	Web	Inside Flange
Col (RC-27)	5/16 x 8 x 15-0 5/16 x 8	3/16 x 12 to 29 15/16 x 15-0 5/16 x 29 15/16 to 36	5/16 x 8 x 15-0 5/16 x 8
Col (IC-31)	W 10 x 33		
Col (RC-29)	5/16 x 8 x 15-0 5/16 x 8	3/16 x 12 to 29 15/16 x 15-0 5/16 x 29 15/16 to 36	5/16 x 8 x 15-0 5/16 x 8
Raf (RR-33)	1/4 x 6 3/8 x 6 x 10-0 3/8 x 6 x 14-11 1/2	1/4 x 29 1/2 3/16 x 29 1/2 x 10-0 8 GA x 29 1/2 x 14-11 1/2	3/8 x 6 5/16 x 6 x 10-0 5/16 x 6 x 14-11 1/2
Raf (RR-35)	1/4 x 6 x 14-8 7/8	1/4 x 29 1/2 to 34 x 14-8 7/8	1/4 x 6 x 14-8 7/8
Raf (RR-39)	1/4 x 6 x 4-2 3/4 1/4 x 6 x 4-2 3/4	1/4 x 34 to 38 x 4-2 3/4 1/4 x 38 to 34 x 4-2 3/4	1/4 x 6 x 4-2 3/4 1/4 x 6 x 4-2 3/4
Raf (RR-41)	1/4 x 6 3/8 x 6 x 10-0 3/8 x 6 x 14-11 1/2	1/4 x 29 1/2 3/16 x 29 1/2 x 10-0 8 GA x 29 1/2 x 14-11 1/2	3/8 x 6 5/16 x 6 x 10-0 5/16 x 6 x 14-11 1/2
Raf (RR-43)	1/4 x 6 x 14-8 7/8	1/4 x 29 1/2 to 34 x 14-8 7/8	1/4 x 6 x 14-8 7/8



Main Frame Cross Section at Frame Lines: frame line 11 (N.T.S)

1/2 x 8 x 3-1 1/2 Plate With (16) 3/4 Dia. x 2 A325	1/2 x 6 x 3-1 1/2 Plate With (10) 3/4 Dia. x 2 A325	1/2 x 6 x 3-6 Plate With (10) 3/4 Dia. x 2 A325	3/8 x 8 x 0-9 3/4 Plate With (4) 3/4 Dia. x 1 3/4 A325

DRAWING STATUS	
☐ For Approval:	
☒ For Construction Permit:	
☐ For Erector Installation:	

ISSUE	DATE	DESCRIPTION	BY	CHKD
A	8/31/07	FOR CONSTRUCTION PERMIT	JAC	DGW

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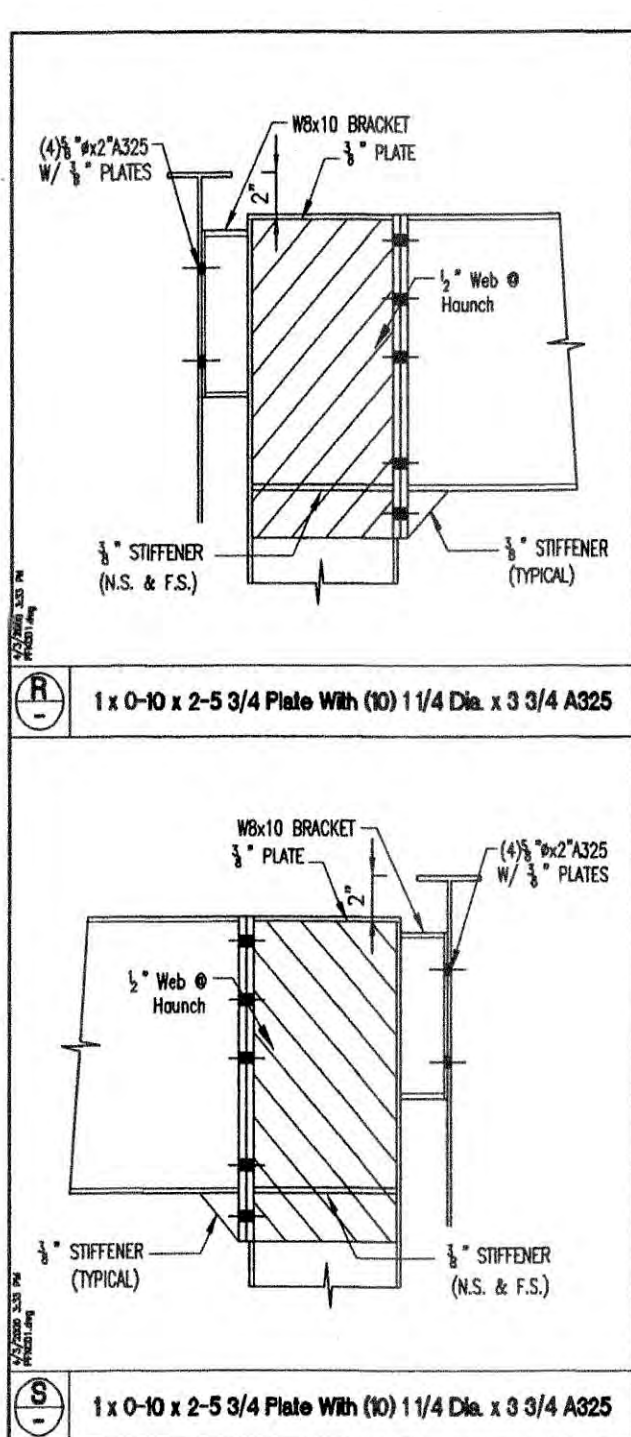
METALLIC metallic building company

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ZIP 77041 (713) 466-7788 ZIP 77240

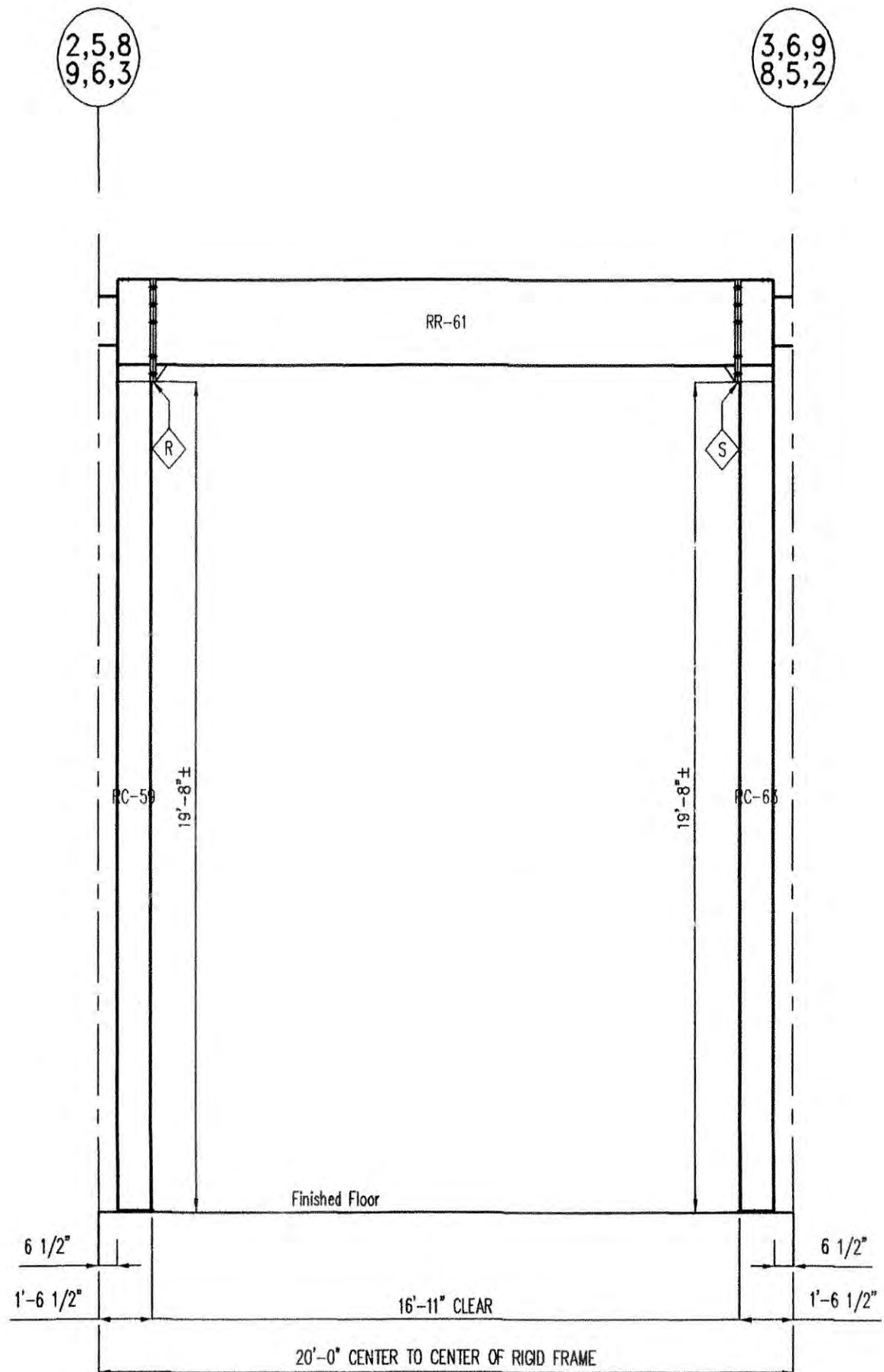
Project: TRUCK SHOP / OFFICE
Customer: HEDGES CREEK MANAGEMENT # 1 LLC Owner: HEDGES CREEK MANAGEMENT # 1 LLC
Location: SCAPOOSE, OR

CAD	Date	Scale	Phase	Building ID	Job Number	Sheet Number	Issue
		N.T.S.	-	A	0805-236857	9 of 11	A

9164
OREGON
JULY 22, 1971
BERT S. HOEKSTRA



MEMBER DATA			
Column data is listed from Base to Eave - Rafter data is listed from Eave to Ridge (High Side)			
Mark	Outside Flange	Web	Inside Flange
Col (RC-59)	1/2 x 0-10	1/4 x 11	1/2 x 0-10
Col (RC-63)	1/2 x 0-10	1/4 x 11	1/2 x 0-10
Raf (RR-61)	1/2 x 8	3/16 x 24	1/2 x 8



PORTAL FRAME AT LINES A&G

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DRAWING STATUS		ISSUE	DATE	DESCRIPTION	BY	CHKD
☐	For Approval:	A	8/31/07	FOR CONSTRUCTION PERMIT	JAC	DGW
☒	For Construction Permit:					
☐	For Erector Installation:					

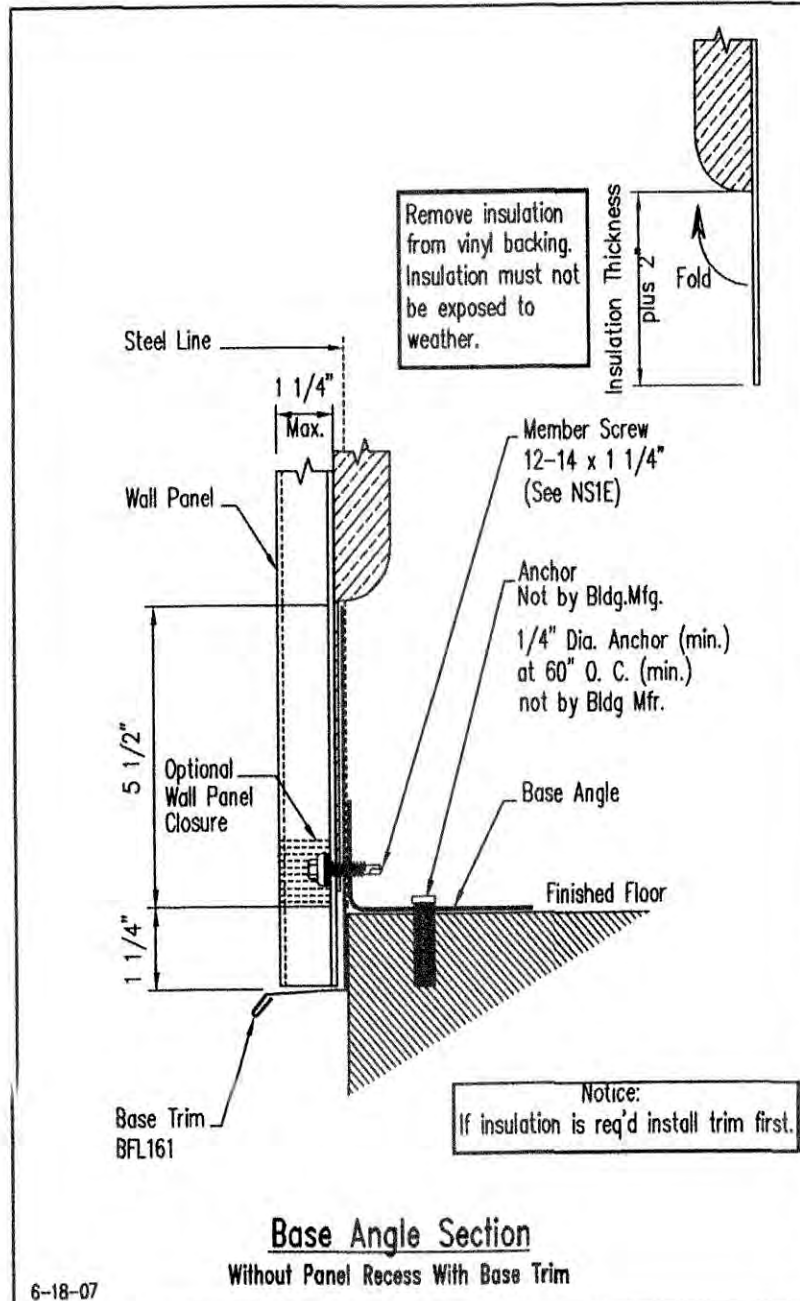
METALLIC		metallic building company	
7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338		7301 FAIRVIEW, HOUSTON, TEXAS, P.O. BOX 40338	
ZIP 77041 (713) 486-7788		ZIP 77041 (713) 486-7788	
Project: TRUCK SHOP / OFFICE			
Customer: HEDGES CREEK MANAGEMENT # 1 LLC		Owner: HEDGES CREEK MANAGEMENT # 1 LLC	
Location: SCAPPOSE, OR			
CAD	Date	Scale	Phase
		N.T.S.	-
Building ID	Job Number	Sheet Number	Issue
A	0805-236857	E11 OF 11	A

The engineer whose seal appears hereon is not the engineer of record for the overall project.

9164

ORIGON JULY 22, 1984

ROBERT S. HOEKSTRA



Nominal Bolt Size, inches	Minimum Tension ^a in 1000's of Pounds (kips)	
	A325 Bolts	A490 Bolts
1/2	12	15
5/8	19	24
3/4	28	35
7/8	39	49
1	51	64
1 1/8	56	80
1 1/4	71	100
1 3/8	85	121
1 1/2	103	148

^aEquals to 70 percent of specified minimum tensile strength of bolts (as specified in ASTM Specifications for tests of full size A325 and A490 bolts with UNC threads loaded in axial tension) rounded to the nearest kip.

Table 4. Fastener Tension Required for Slip-critical Connection & Connections Subject to Direct Tension

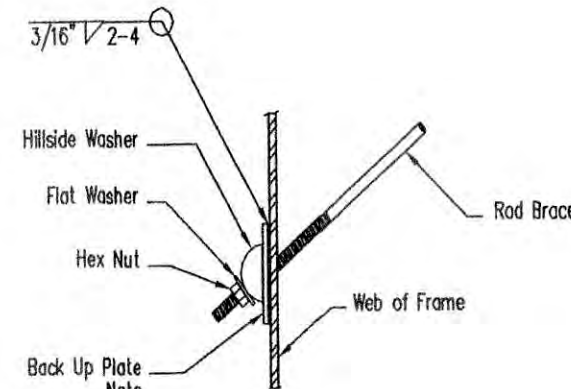
Bolt length (under side of head to end of bolt)	Disposition of Outer Face of Bolted Parts		
	Both faces normal to bolt axis	One face normal to bolt axis and other sloped not more than 1:20 (beveled washer not used)	Both faces sloped not more than 1:20 from normal to the bolt axis (beveled washer not used)
Up to and including 4 diameters	1/3 turn	1/2 turn	2/3 turn
Over 4 diameters but not exceeding 8 diameters	1/2 turn	2/3 turn	5/6 turn
Over 8 diameters but not exceeding 12 diameters	2/3 turn	5/6 turn	1 turn

^bNut rotation is relative to bolt regardless of the element (nut or bolt) being turned. For bolts installed by 1/2 turn and less, the tolerance should be plus or minus 30 degrees; for bolts installed by 2/3 turn and more, the tolerance should be plus or minus 45 degrees.

^cApplicable only to connections in which all material within the grip of the bolt is steel.

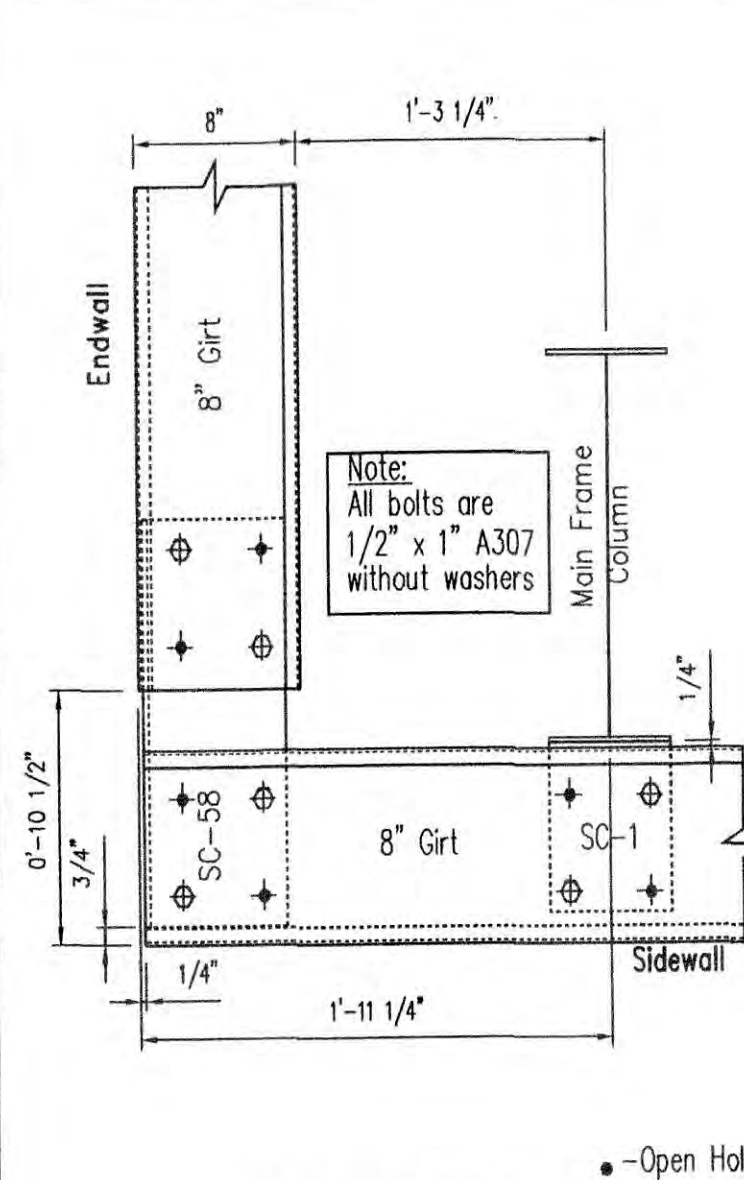
^dNo research has been performed by the Council to establish the turn-of-nut procedure for bolt lengths exceeding 12 diameters. Therefore, the required rotation must be determined by actual test in a suitable tension measuring device which simulates conditions of solidly fitted steel.

Table 5. Nut Rotation from Snug Tight Condition^{a,b}

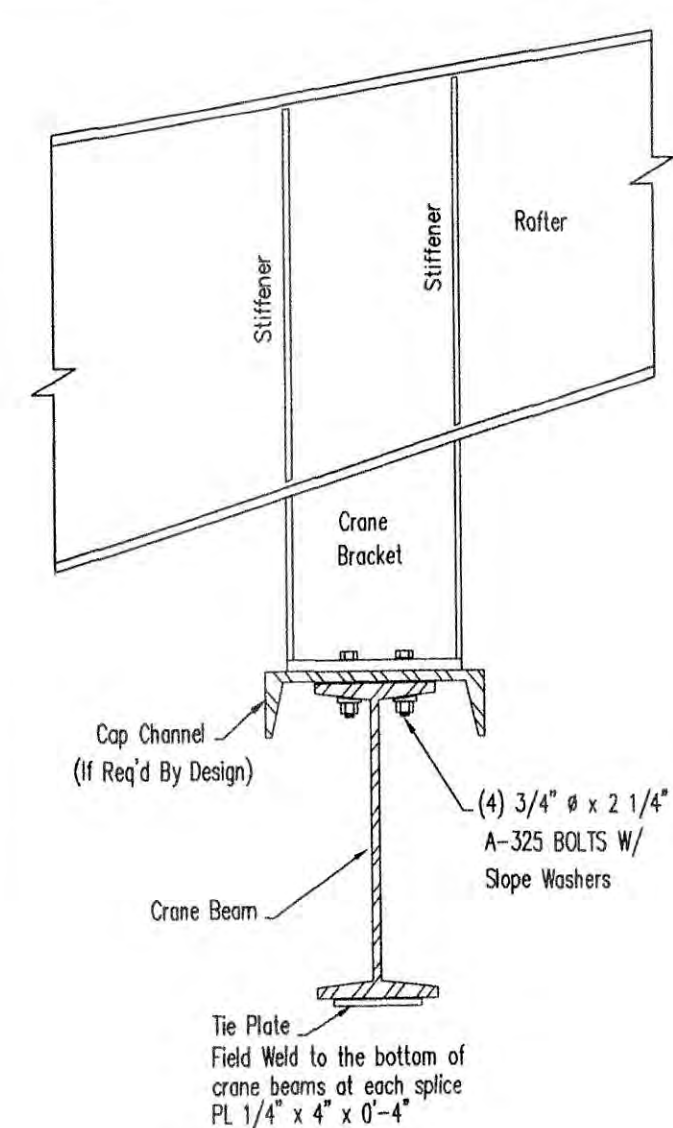


Rod Brace to Frame Connection

Note: Shop welded Back Up Plate may be located at either side of the web. (If Required By Design)



Corner Detail For Bypass Sidewall-Bypass Endwall



UNDER HUNG CRANE BRACKET DETAIL

NS0062E

CONNECTION BOLTS

Bolts used to make connections in primary framing members such as columns and rafters are usually ASTM A325 bolts. Bolts used to make connections in secondary framing members such as the purlins are usually ASTM A307 bolts. The size and grade of the bolts are marked on the Building Construction Drawings.

The following information is copied from the AISC ninth edition, "Specification For Structural Joints Using ASTM A325 or A490 Bolts". This is provided to you only as a guide. It is the responsibility of you as the installer to assure proper tightness of all bolts.

TURN-OF-NUT TIGHTENING

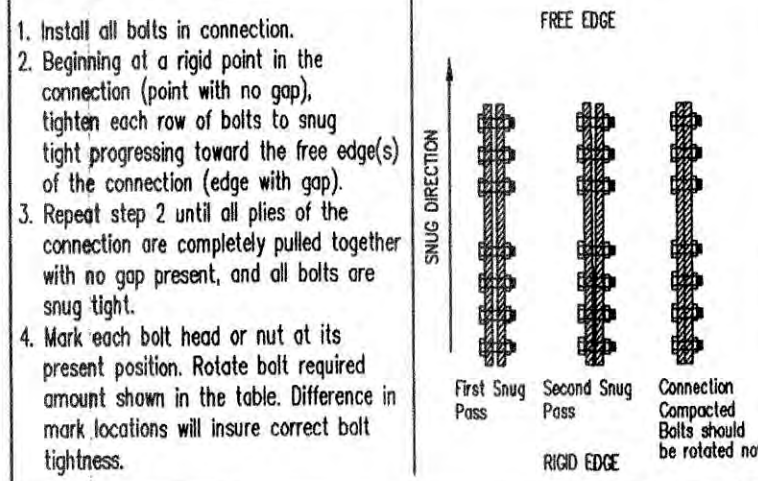
When turn-of-nut tightening is used, hardened washers are not required except (as otherwise indicated on the Construction Drawings.)

A representative sample of not less than three bolts and nuts of each diameter, length, and grade to be used in the work shall be checked at the start of work in a device capable of indicating bolt tension. The test shall demonstrate that the method of estimating the snug-tight condition and controlling turns from snug tight to be used by the bolting crews develops a tension not less than five percent greater than the tension required by Table 4.

Bolts shall be installed in all holes of the connection and brought to a snug tight condition. Snug tight is defined as the tightness that exists when the plies of the joint are in firm contact. This may be attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Snug tightening shall progress systematically from the most rigid part of the connection to the free edges, and then the bolts of the connection shall be retightened in a similar systematic manner as necessary until all bolts are simultaneously snug tight and the connection is fully compacted. Following the initial operation all bolts in the connection shall be tightened further by the applicable amount of rotation specified in Table 5. During the tightening operation there shall be no rotation of the part not turned by the wrench. Tightening shall progress systematically from the most rigid part of the joint to its free edges. Impact wrenches, if used, shall be of adequate capacity and sufficiently supplied with air to perform the required tightening of each bolt in approximately 10 seconds.

Bolt Tightening

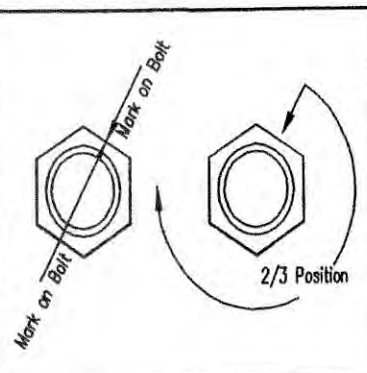
NS0238A



Turn-of- Nut Sequence Order

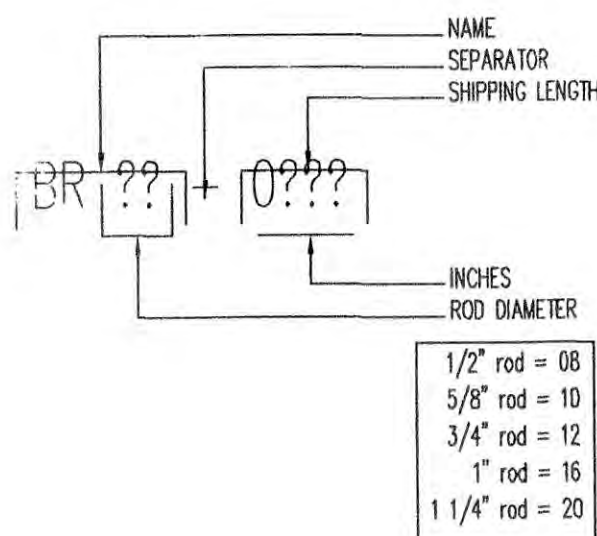
When connection is compacted and bolt is snug:

1. Mark nut and bolt location with keel or chalk.
2. Rotate nut as required per chart conditions.
3. Finished mark locations are proof that bolts have been properly tightened.



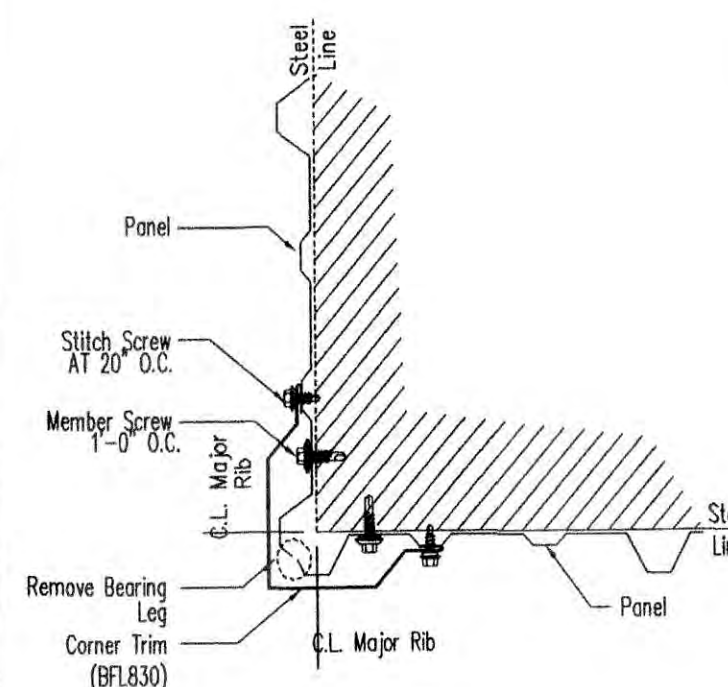
Turn-of- Nut Procedure

NS0053A



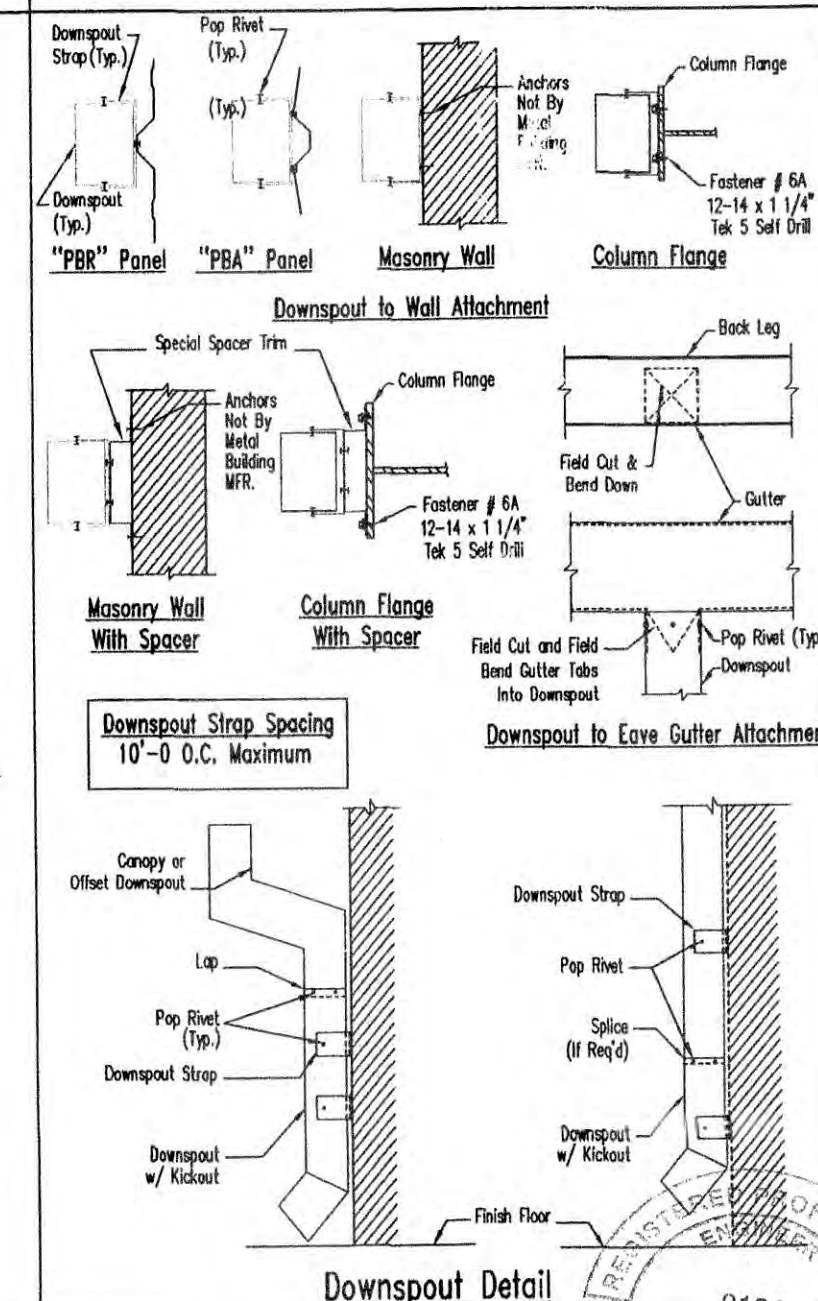
ROD BRACING INFORMATION

NS0983



"PBR" Panel Outside Corner Trim

NS0047A



Downspout Detail

NS0238

NOTE:
1. These drawings are intended to depict general installation of item(s) described above. Some item(s) may have been omitted for clarity of presentation. Consult your erection manual or additional S-Sheets for further guidelines and/or clarifications.
ATTENTION INSTALLER: In the event that a discrepancy or error arises with materials shipped for this project or these construction drawings, the installer must notify the manufacturer prior to correcting. If manufacturer is not notified, manufacturer will

NS0238B

NS0053D

NS0015

NS0009

REVISIONS				
NO.	DATE	DESCRIPTION	BY	CK'D
A		FOR CONSTRUCTION PERMIT		

Standard Details									
Customer: HEDGES CREEK MANAGEMENT # 1 LLC									
End User: HEDGES CREEK MANAGEMENT # 1 LLC									
Location: ACAPPASE, OR									
Det. By	Chk By	Date	Scale	Building ID	Job Number	Sheet Number	Issue		
			N.T.S.	A	0805-236857	NS-1	A		

The diagram illustrates the layout of edge zone purlins. The main side elevation on the left shows a vertical section with the following components from top to bottom: Purlin, Zone Purlin, Purlin, Zone Purlin, and Strut Purlin. The purlins are connected by SC-46 clips. The edge zone members are labeled with dimensions: 8", 10", and 12" Cee. The layout is divided into three detail areas: Detail "C" at the top, Detail "B" in the middle, and Detail "A" at the bottom. The bottom section is labeled "Eave Strut" and "Edge Zone as per design". A note in a box at the top left states: "All bolts shown are 1/2" X 1" A307 Bolts w/o washers".

Detail "C"

Detail "C" shows a cross-section of a purlin connected to an edge zone member (labeled "Edge Zone Member 8", 10", 12" Cee") using an SC-46 clip. The connection is secured with bolts. The label "Detail 'C'" is underlined.

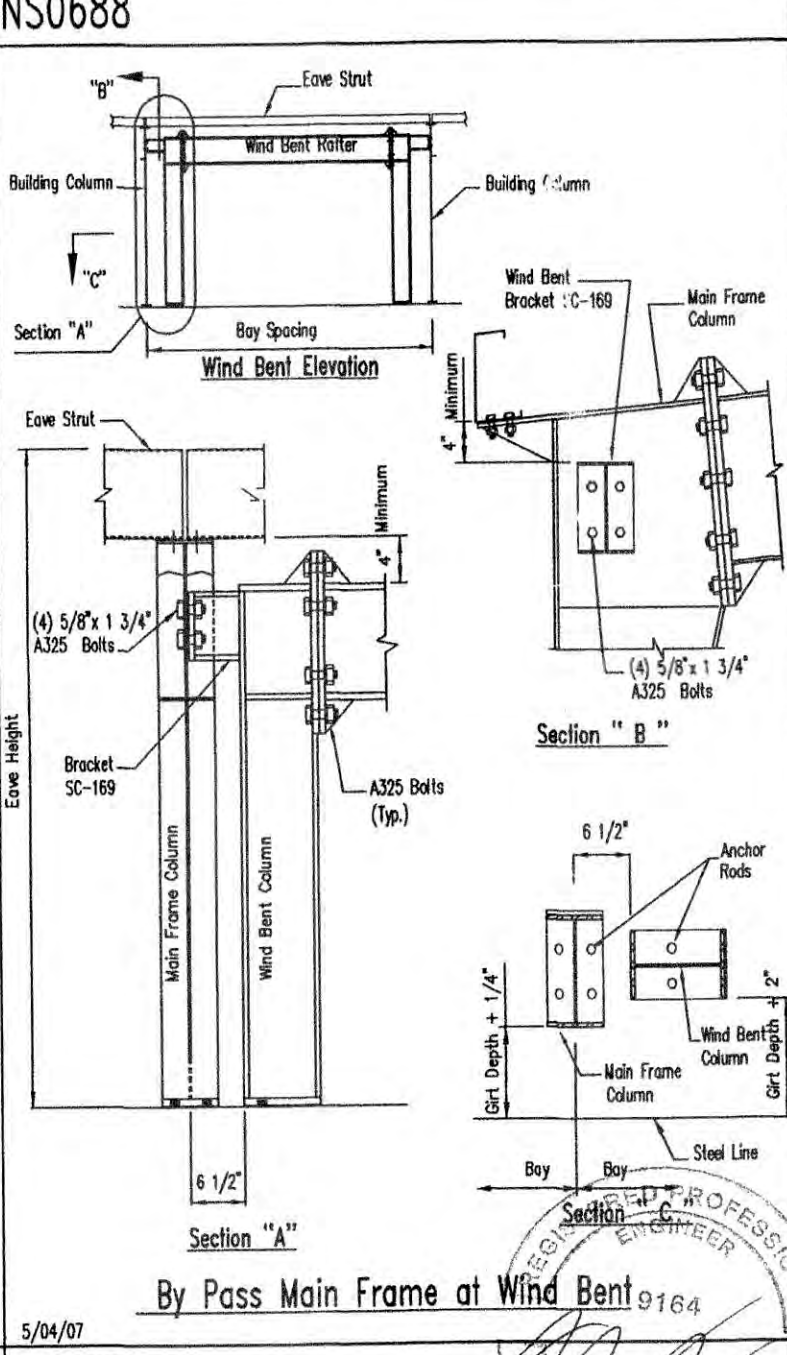
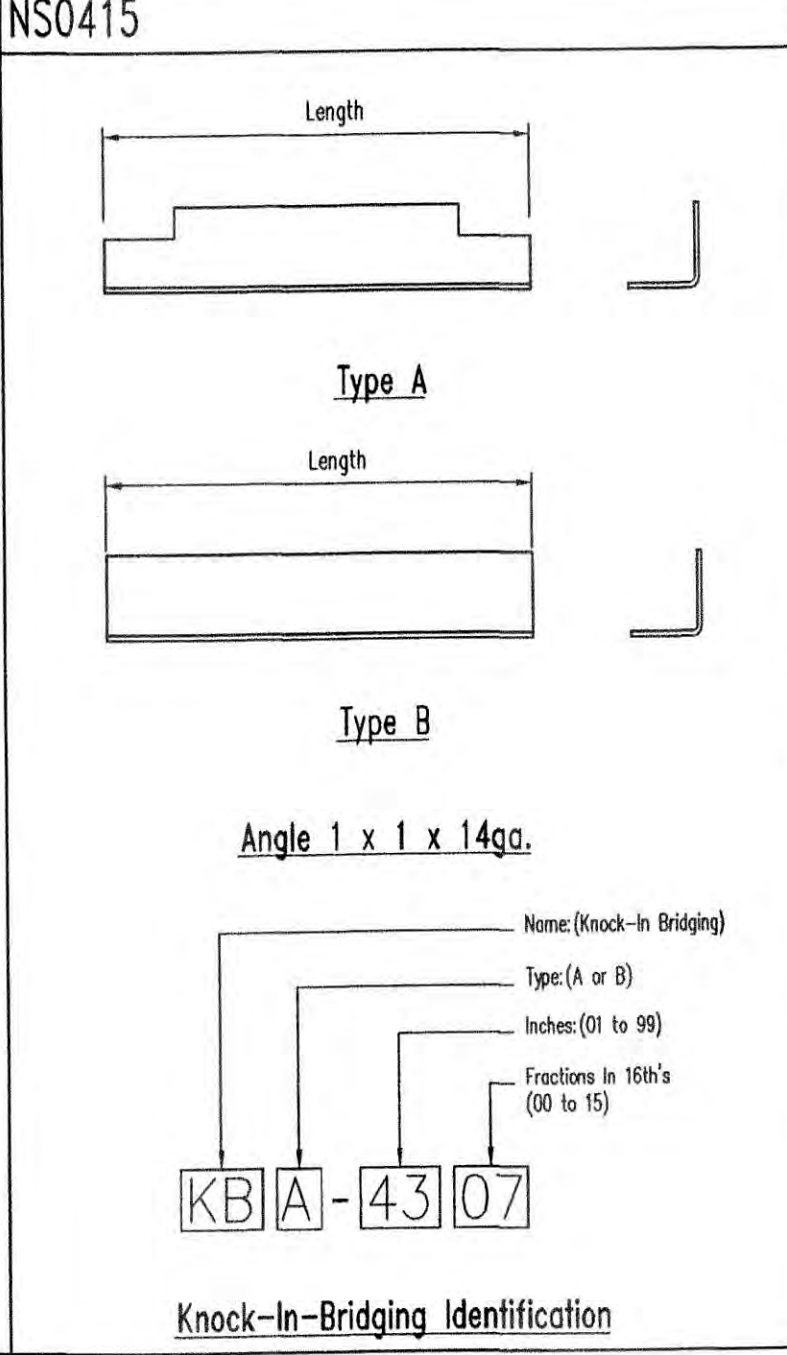
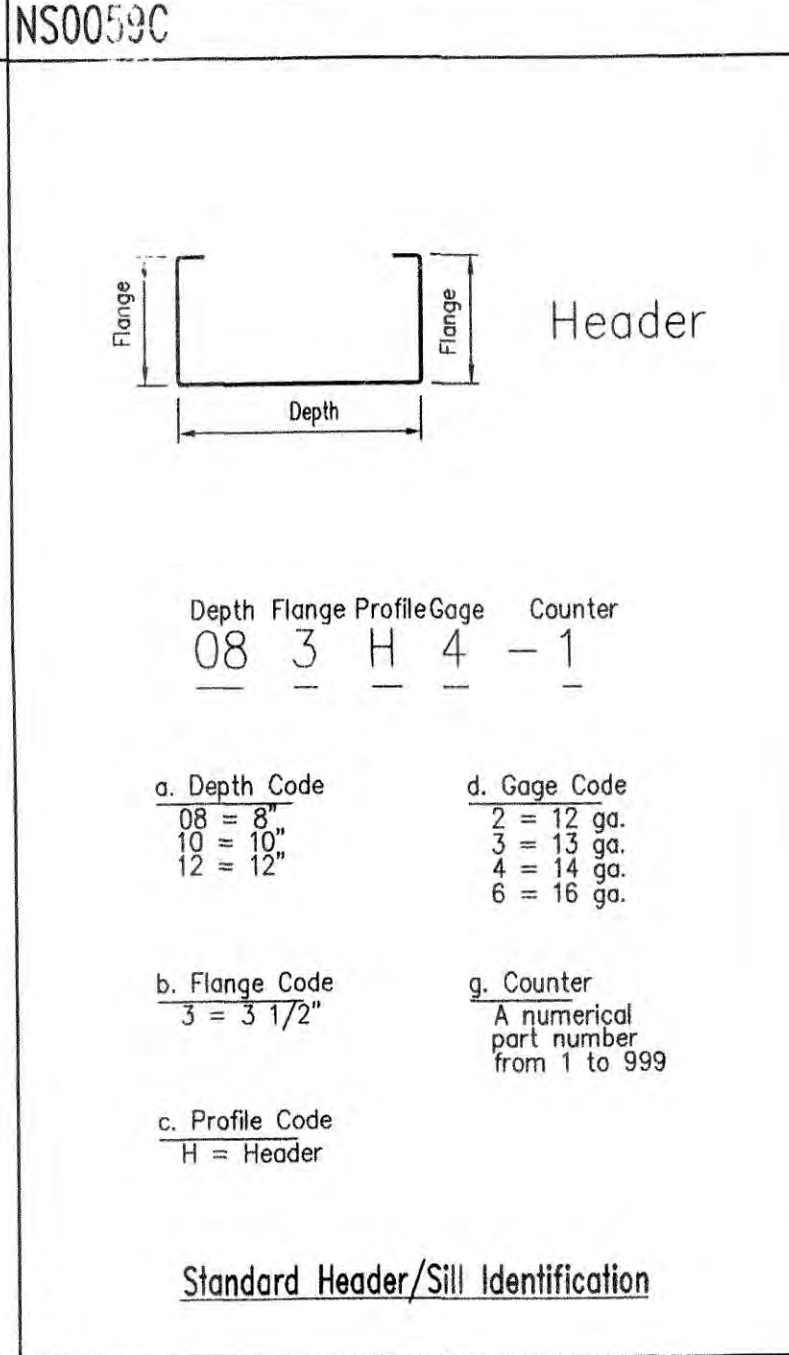
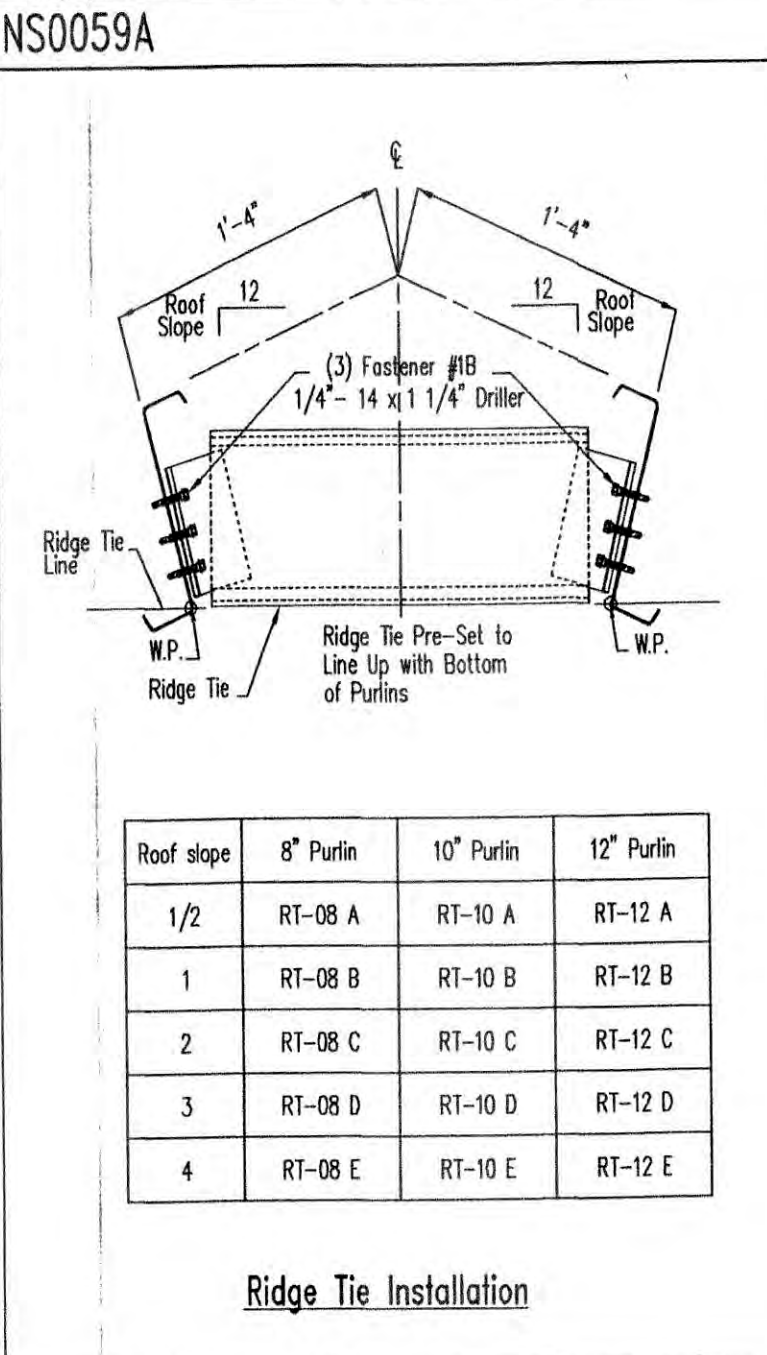
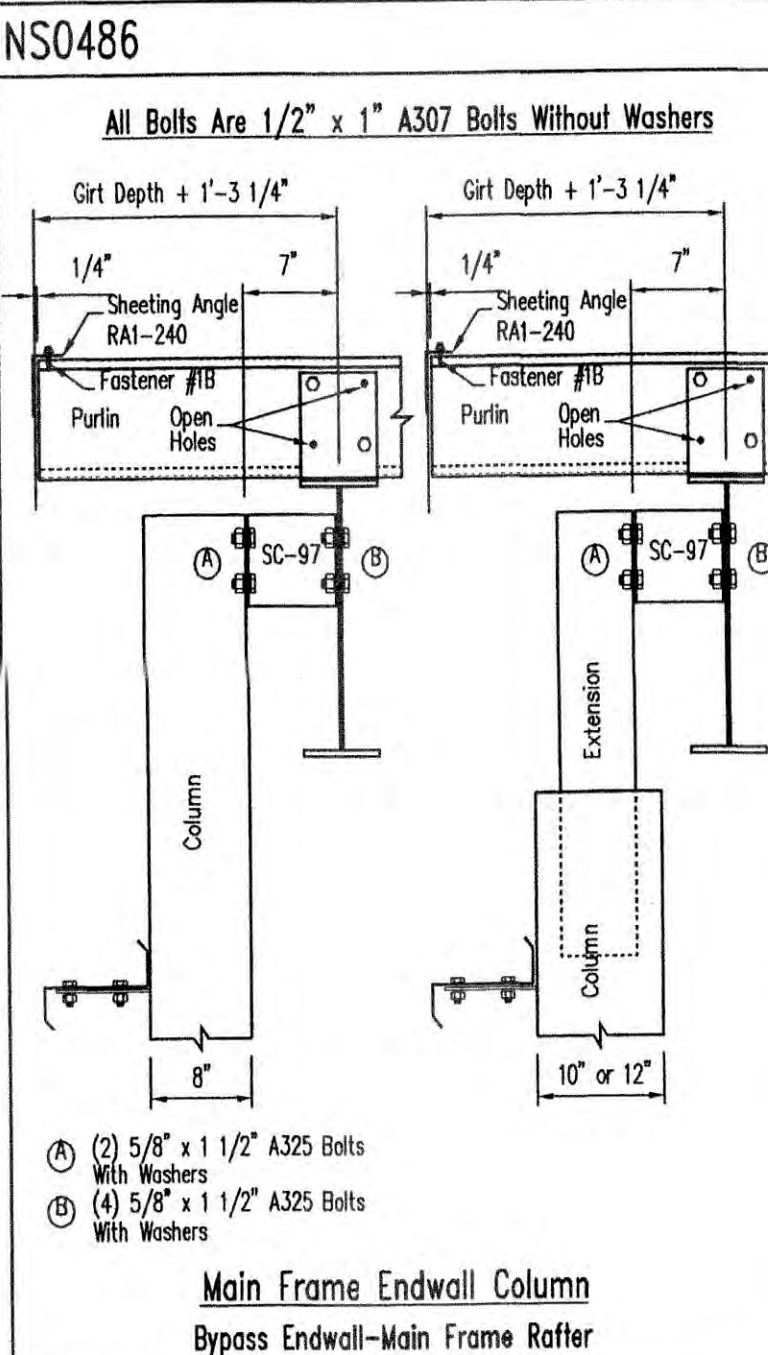
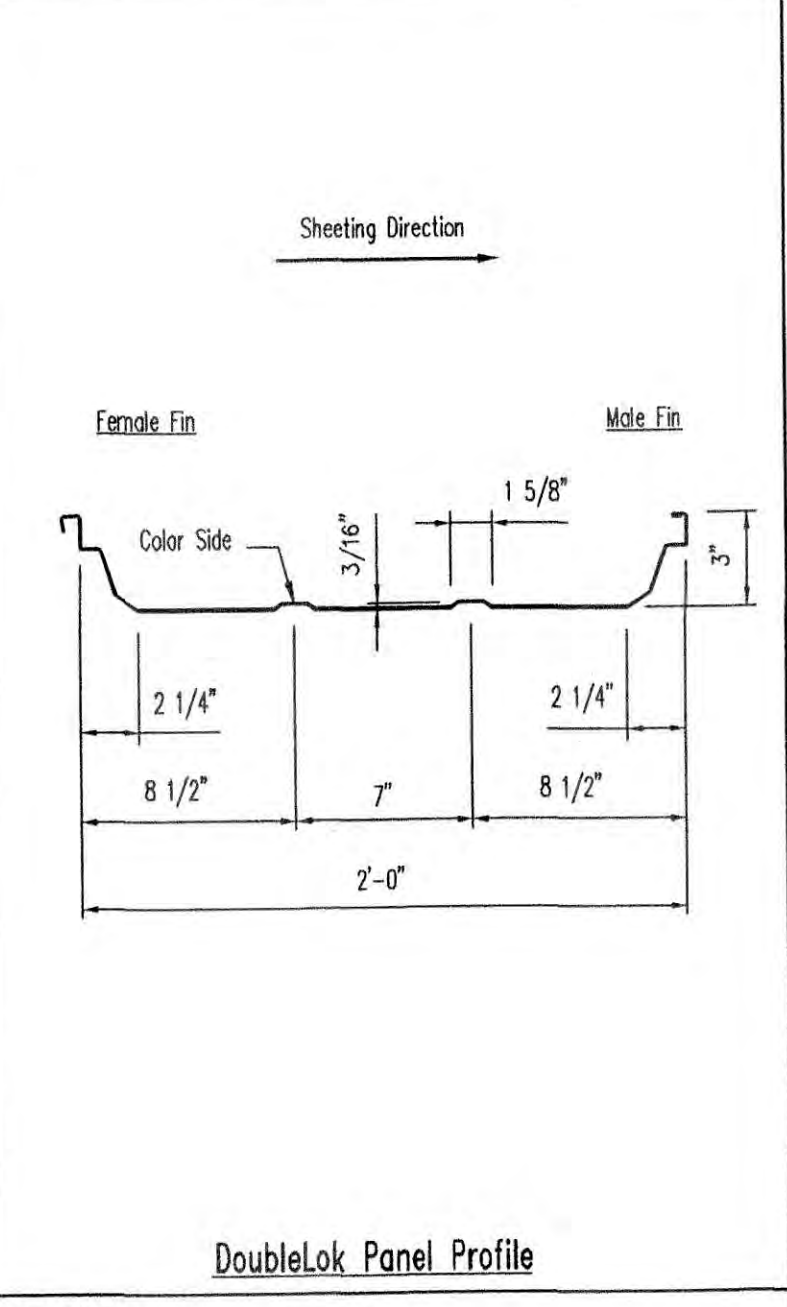
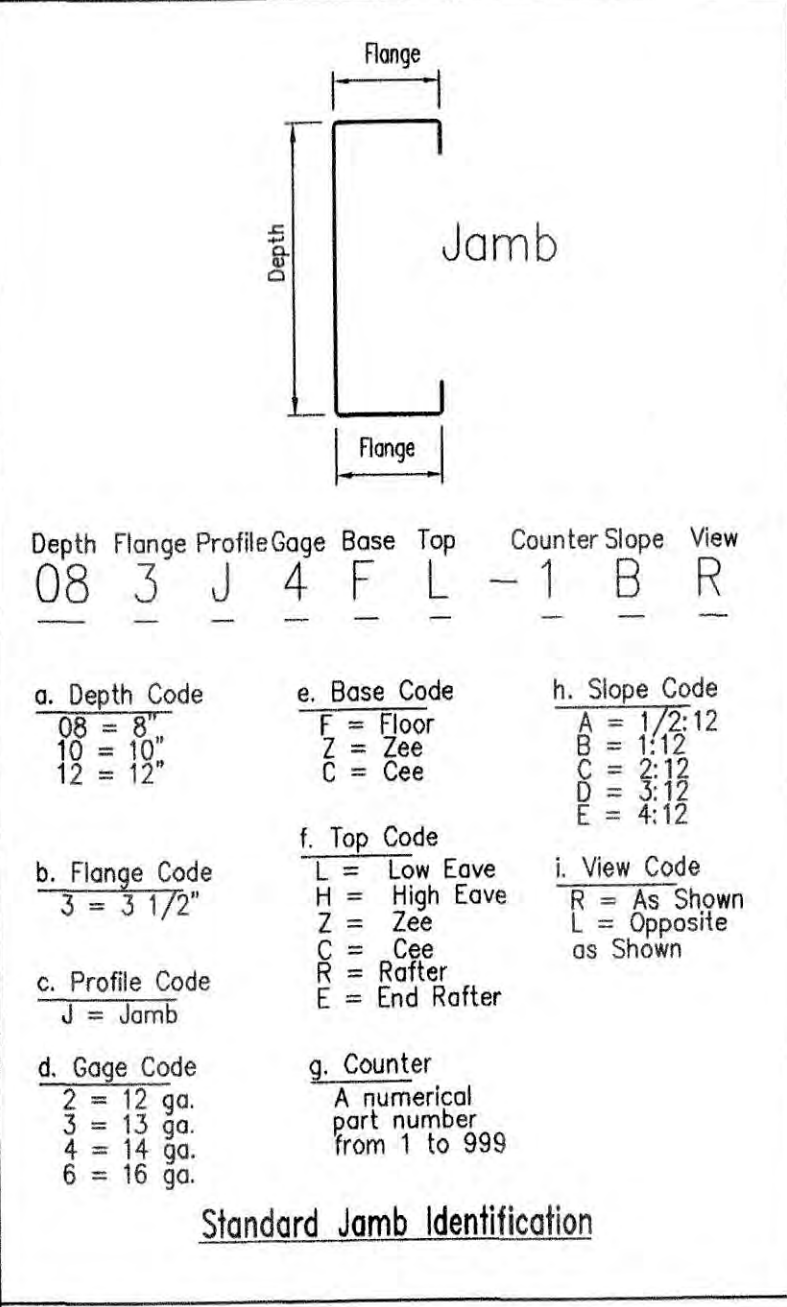
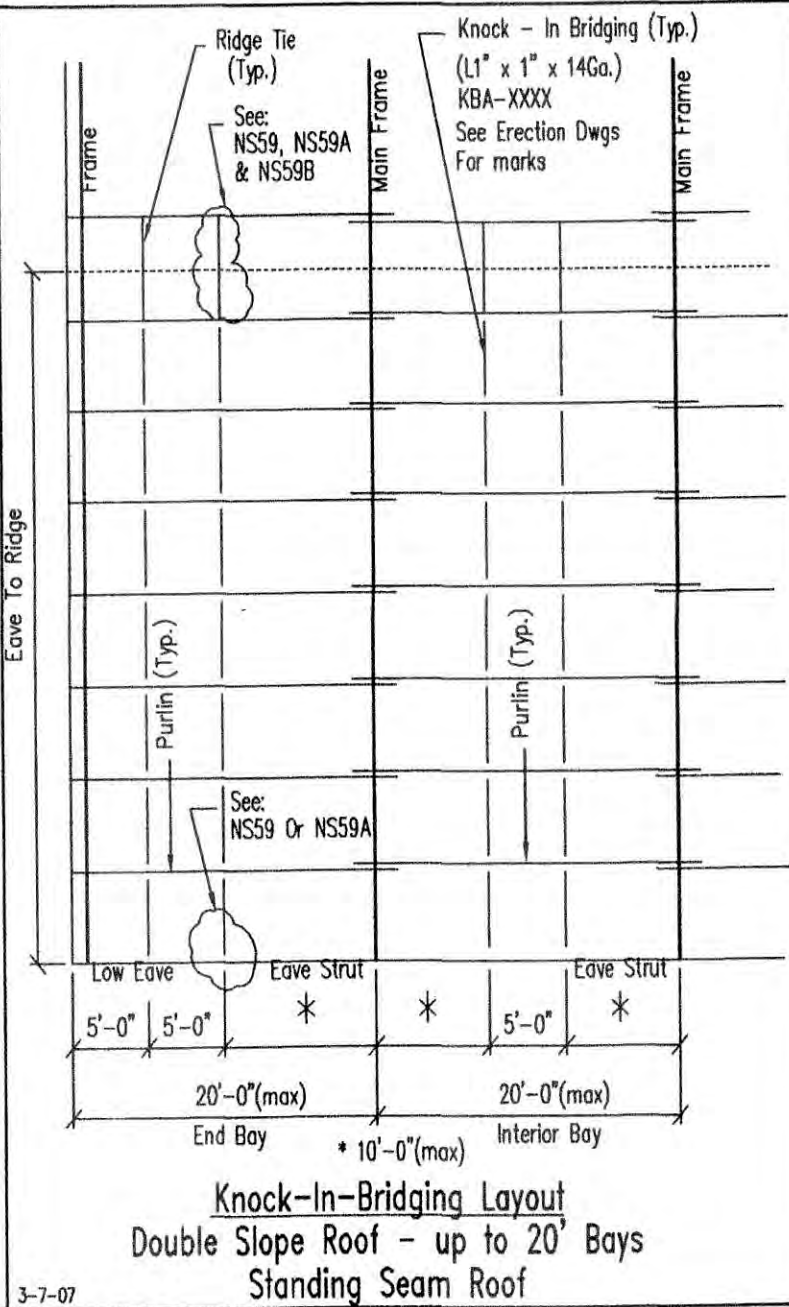
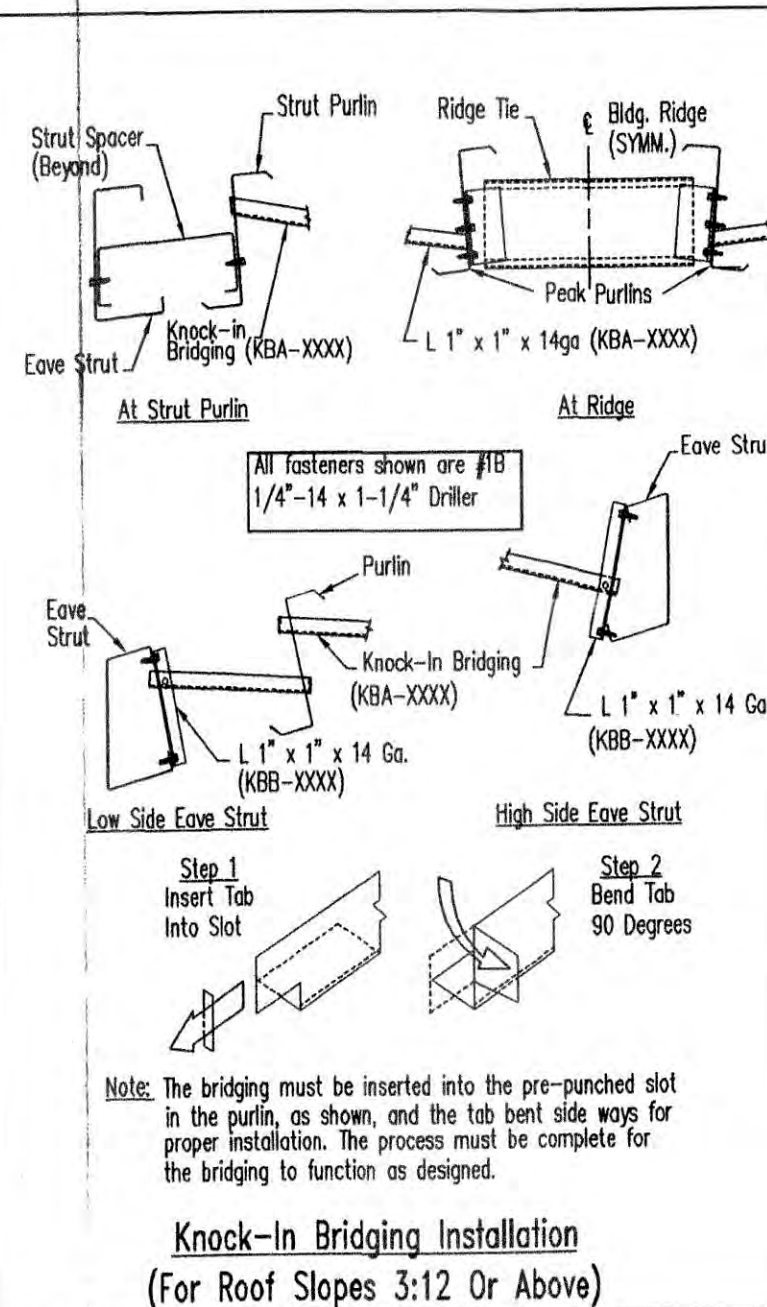
Detail "B"

Detail "B" shows a cross-section of a purlin connected to an edge zone member (labeled "Edge Zone Member 8", 10", 12" Cee") using an SC-46 clip. The connection is secured with bolts. The label "Detail 'B'" is underlined.

Detail "A"

Detail "A" shows a cross-section of a purlin connected to an edge zone member (labeled "Edge Zone Member 8", 10", 12" Cee") using an SC-46 clip. The connection is secured with bolts. The label "Detail 'A'" is underlined.

Edge Zone Purlin Layout



NS0013A	NS0059B	NS0416	NS0059J	NS0052A																																																										
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FASTENER #1 1/4"-14 x 1" Driller 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #1E 1/4"-14 x 1 1/4" Long Life Driller 5/16" Hex Washer Head w/ Sealing Washer	FASTENER #3 12-14 x 1 1/4" Long Life Driller 5/16" Hex Washer Head w/ Sealing Washer	FASTENER #32 14 x 1 1/2" TYPE AB Long Life 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #44 1/4"-14 x 7/8" Lap TEK 5/16" Hex Washer Head w/ 1 1/8" O.D. Washer	FASTENER #226 3/16" x 9/16" Closed End Rivet
FASTENER #1A 12-14 x 1" Driller 5/16" Hex Washer Head	FASTENER #1F 1/4"-14 x 1 1/2" Driller 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #3A 12-14 x 1 1/2" Long Life Driller 5/16" Hex Washer Head w/ Sealing Washer	FASTENER #33 14 x 1 1/4" TYPE AB Long Life 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #44L 1/4"-14 x 7/8" Long Life Lap TEK 5/16" Hex Washer Head w/ 1 1/8" O.D. Washer	FASTENER #228 10 x 1/2" Grommet Washer
FASTENER #1B 1/4"-14 x 1 1/4" Driller 5/16" Hex Washer Head	FASTENER #2A 17 x 1" Long Life AB 5/16" Hex Washer Head w/ Sealing Washer	FASTENER #4 1/4"-14 x 7/8" Long Life Lap TEK 5/16" Hex Washer Head w/ Sealing Washer	FASTENER #43 1/4"-14 x 1 1/4" Driller 5/16" Hex Washer Head w/ 1 1/8" O.D. Washer	FASTENER #46 1/4"-14 x 5/8" Long Life Type B 5/16" Hex Washer Head w/ Sealing Washer	FASTENER #HW399 #6 x 1" Rubber Grommet 1/4" Hex Head w/ Washer
FASTENER #1C 1/4"-14 x 3/4" Driller 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #2B 1/4"-14 x 1" Long Life AB 5/16" Hex Washer Head w/ Sealing Washer	FASTENER #4A 1/4"-14 x 7/8" Lap TEK 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #43L 1/4"-14 x 1 1/4" Driller 5/16" Hex Washer Head w/ 1 1/8" O.D. Washer	FASTENER #66 14 x 3/4" Type AB Long Life 5/16" Hex Washer Head w/ 5/8" O.D. Washer	

Self-drilling fasteners are designed to drill the material and tap the hole. Material thickness is an important consideration when determining the proper point diameter. The point diameter is described as the point TYPE. Point types are as follows:

TYPE 1: Light gauge application
TYPE 2: Medium light gauge application
TYPE 3: Medium gauge application
TYPE 4: Medium heavy gauge application
TYPE 5: Heavy gauge application

The point diameter allows the threads to tap into the material without excessive force which can break the shank, or very low force which can easily strip-out the material during seating torque. Each point type has a specific FLUTE length which allows the chip to escape from the hole during the drilling operation. Depending on the application, some drill screws require a special PILOT section. The pilot section is the area that is unthreaded on the drill point. The pilot area is important to the performance of the fastener. The reason for the pilot area is so that the drilling of the hole is completed before the threads engage. If the threads engage before the drilling is completed, the drill point will have a tendency to burn because the threads will force the point into the material.

Material Thickness: Drilling Point Capabilities

Point Type:

Type 1 Light Duty	Type 2 Medium Duty	Type 3 Medium Duty	Type 4 Heavy Duty	Type 5 Heavy Duty
#10 (.035-.090)	#6 (.035-.098)	#8 (.100-.140)	#12 (.145-.312)	#12 (.250-.500)
#12 (.035-.090)	#8 (.035-.100)	#10 (.110-.175)		
1/4" (.035-.050)	#10 (.090-.110)	#12 (.090-.210)		
#18 (.035-.098)	#12 (.050-.140)	1/4" (.110-.250)		

Self-Drilling Fasteners

Standard Grade

Description	Fastener Number	Application
1/4"-14 x 7/8" Type 2	4A	Stitch & Trim Screw
1/4"-14 x 1 1/4" Type 2	17A	Member Screw (Up To 4" Insulation)
1/4"-14 x 1 1/2" Type 2	17B	Member Screw (Up To 6" Insulation)

Long Life

Description	Fastener Number	Application
1/4"-14 x 7/8" Type 1	4	Stitch & Trim Screw
1/4"-14 x 1 1/4" Type 2	3	Member Screw (Up To 4" Insulation)
1/4"-14 x 1 1/2" Type 2	3A	Member Screw (Up To 6" Insulation)

Self-Drilling Screw Application

With Out Liner

With Liner

By-Pass Web Bolted Flange Brace Detail

FASTENER #5 1/4"-14 x 1 1/4" Shoulder TEK 2 5/16" Hex Washer Head	FASTENER #14 1/8" x 5/16" Pop Rivet	FASTENER #17B 12-14 x 1 1/2" Driller 5/16" Hex Washer Head w/ 5/8" O.D. Washer
FASTENER #6 12-24 x 1 1/4" TEK 4.5 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #14A 1/8" x 3/8" Pop Rivet SS	FASTENER #18A 14 x 3/4" TYPE A 5/16" Hex Washer Head w/ 5/8" O.D. Washer
FASTENER #6A 12-24 x 1 1/4" TEK 4.5 5/16" Hex Washer Head	FASTENER #17 12-14 x 1" Driller 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #18B 14 x 1 1/2" TYPE A 5/16" Hex Washer Head w/ 5/8" O.D. Washer
FASTENER #12A 12 x 1 Pancake Self Driller	FASTENER #17A 12-14 x 1 1/4" Driller 5/16" Hex Washer Head w/ 5/8" O.D. Washer	FASTENER #24 1/4"-8 x 5/8" Nibbed Driller

Example:

#12-14 x 1-1/4" Type 3 HWH, Self-Drilling, Carbon Steel, .0005 Minimum Zinc Plate

Labels: Diameter, Length, Head Type, Material, Finish, Threads Per Inch, Point, Fastener Type, Length, Diameter, Point (Long Pilot), Head Type, Threads Per Inch.

Self Drilling Fastener Terminology

The length of the tip required on a drilling fastener is determined by adding the thickness of the top material, any insulation or void between the top material and bottom material, and the thickness of the bottom material. If a hole larger than the screw thread has been pre-drilled or punched into the top material, only the thickness of the bottom material need be considered. (The threaded portion of the shank must be long enough to extend into and through the bottom material drilled.)

Drilling Tip Length Determination

Column

Rafter

Hot Roll or Built Up Connection

Cold Form Connection

Flange Bracing Web Bolted

By Pass Girt

Flush Girt

Flange Brace Detail at Framed Opening

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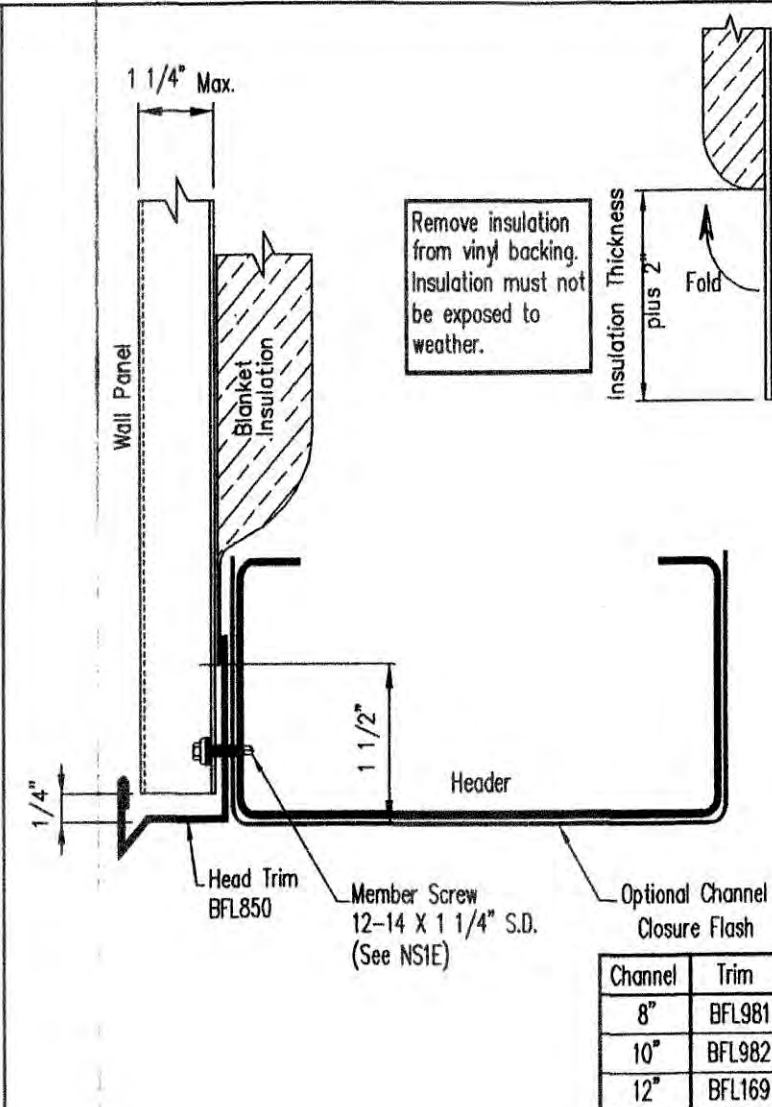
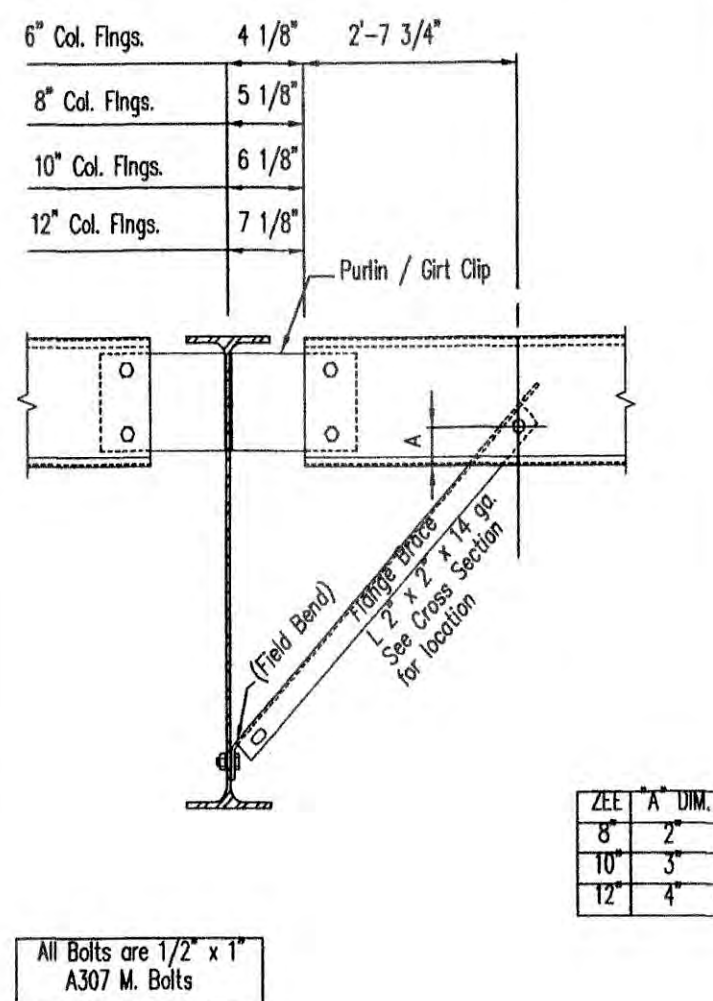
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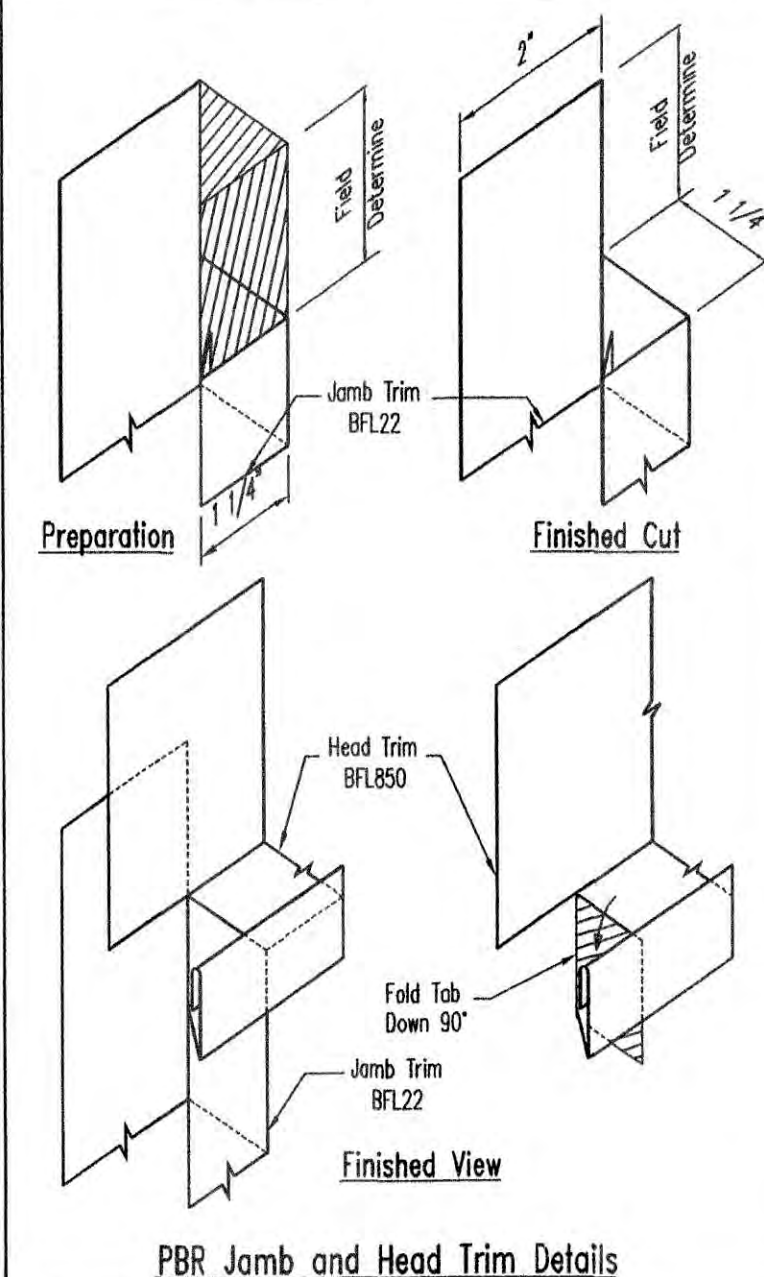
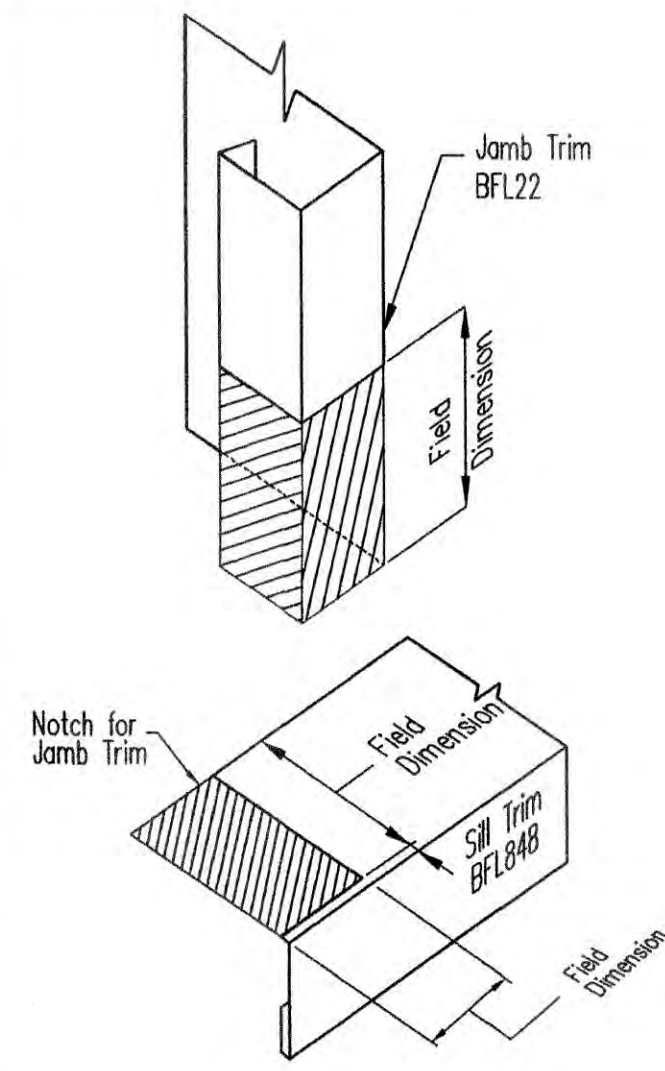
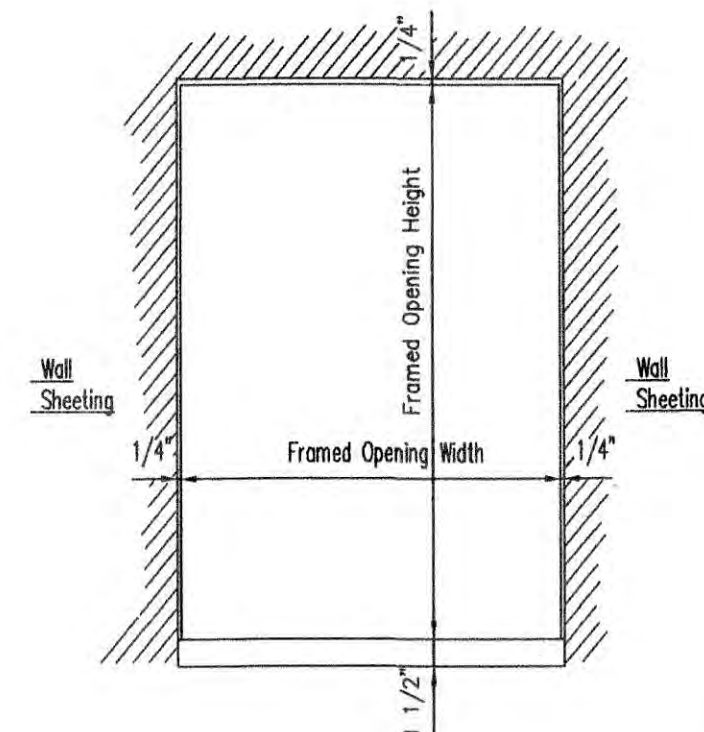
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End User: HEDGES CREEK MANAGEMENT #1 LLC
Location: ACAPPASE, OR

Det. By	Chk By	Date	Scale	Building ID	Job Number	Sheet Number	Issue
			N.T.S.	A	0805-236857	NS-3	A



NOTE:

When field cutting sheeting or trim, do not use abrasive saws or other cutting methods which produce hot metal particles or burn the cut edges. These methods will damage the paint and galvanneal finish and void any warranties. Use double cut shears, nibblers or other cutting devices which do not produce hot metal particles or burned edges.



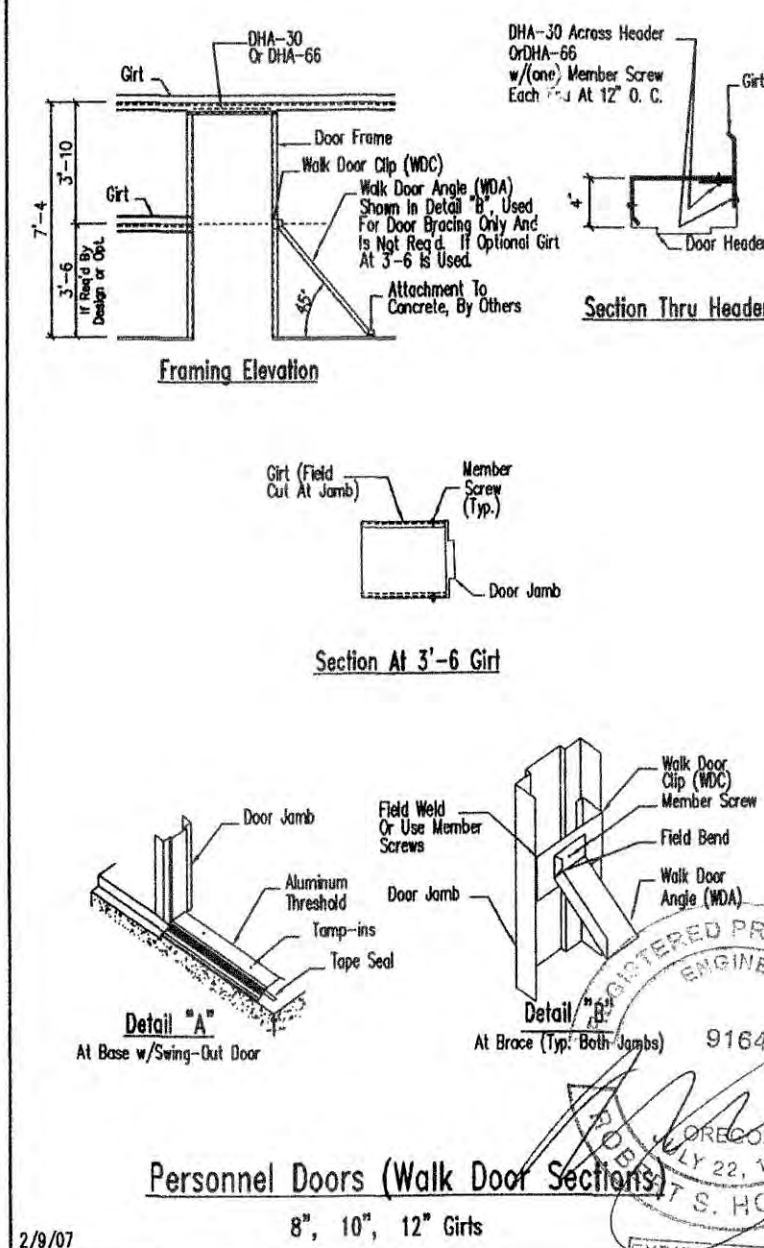
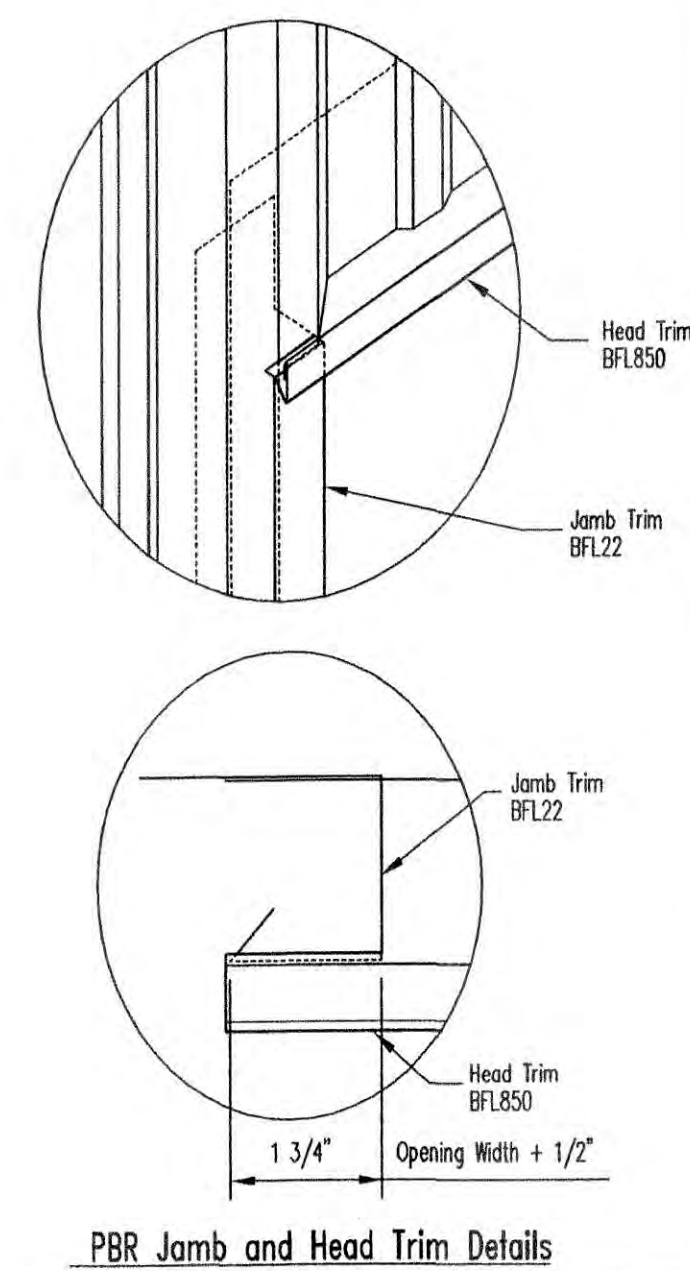
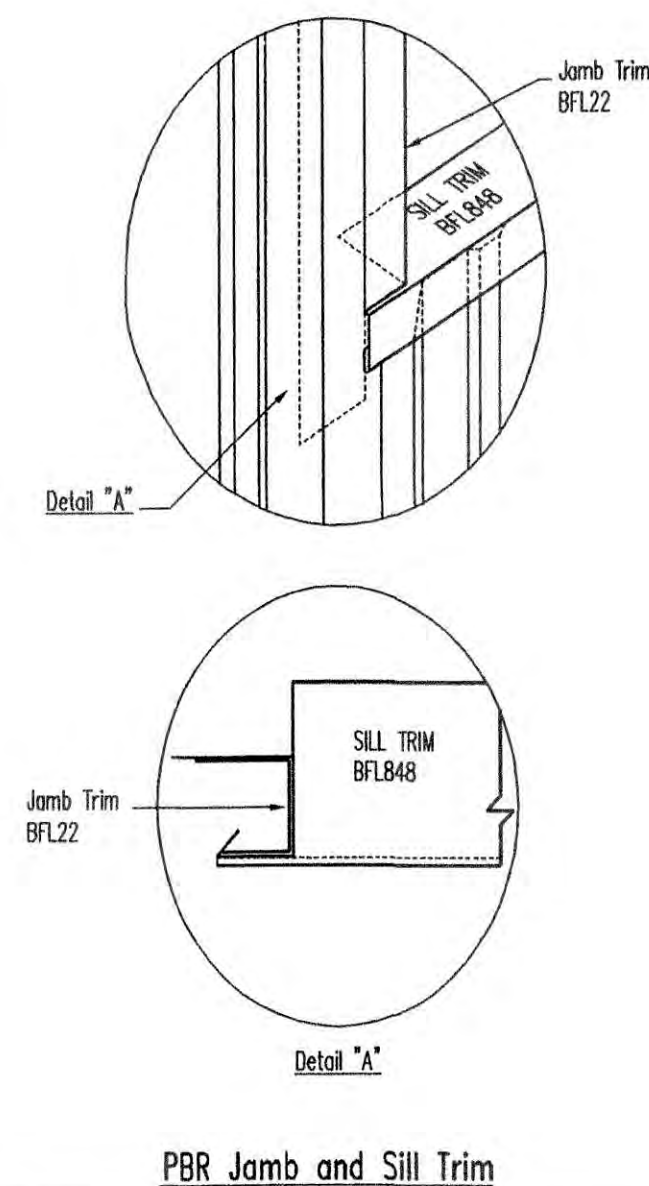
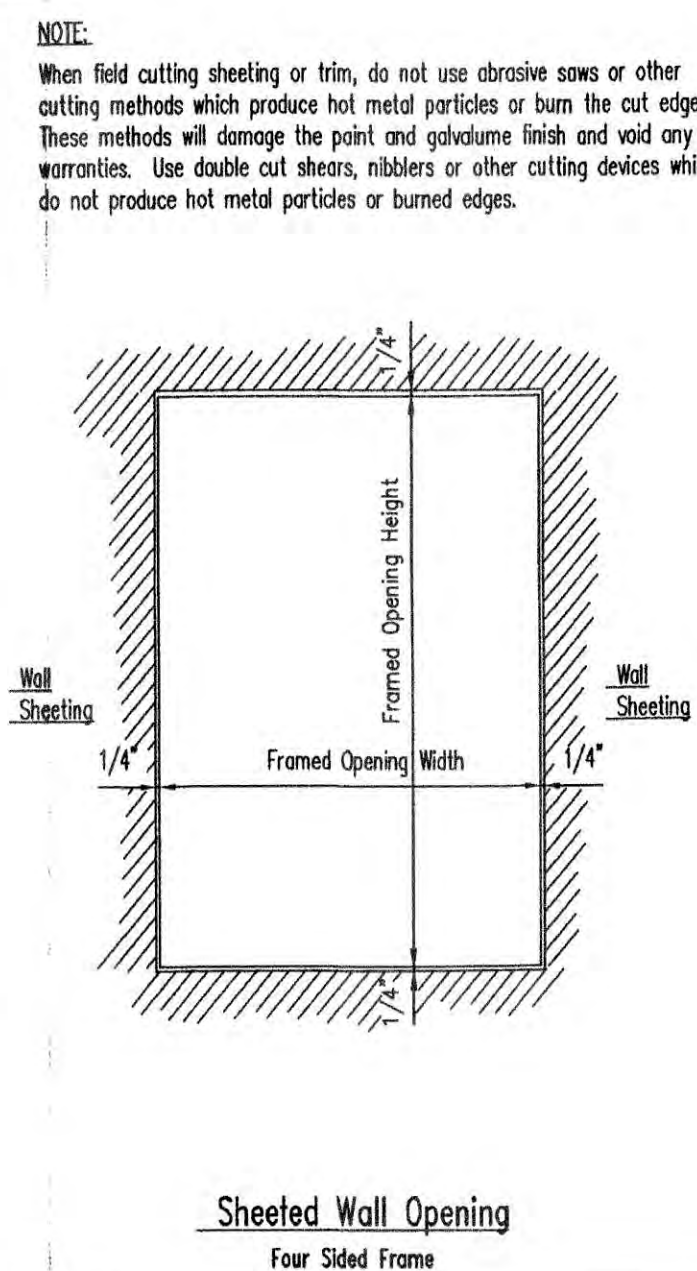
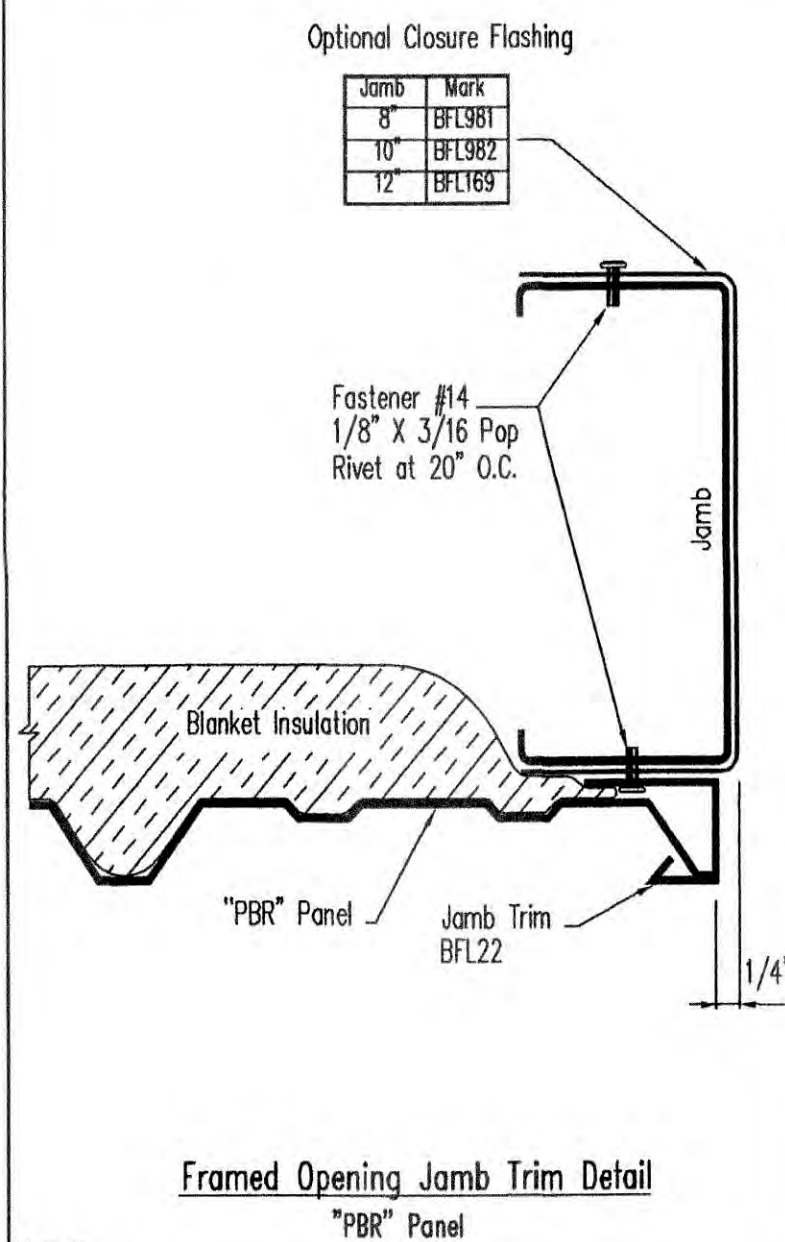
NS0140B

NS0023-1

NS0068D

NS0068F

NS0068H



NS0022C

NS0068C

NS0068E

NS0068G

NS0020

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[illegible]

Standard Details

Customer: HEDGES CREEK MANAGEMENT # 1 LLC							
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Location: ACAPASE, OR							
Del. By	Chk By	Date	Scale	Building ID	Job Number	Sheet Number	Iss
			N.T.S.	A	0805-236857	NS-4	A

Section "A"

Section "B"

Section "C"

Framing Elevation

Personnel Doors (With Jamb Extensions)

Framed Opening with Welded Clips

All connection bolts are 1/2" x 1" mach. bolts

Knock-In Bridging Installation
(For Roof Slopes Below 3:12)

All fasteners shown are #18 1/4" - 14 x 1-1/4" Driller

Knock-In Bridging Layout
Double Slope Roof - up to 20' Bays
Standing Seam Roof

All fasteners shown are #18 1/4" - 14 x 1-1/4" Self Drill (6) Required

Knock-In Bridging Attachment
Low Side Eave Strut

All fasteners shown are #18 1/4" - 14 x 1-1/4" Self Drill (6) Required

Framed Opening With Welded Clips

Jamb	Header
8" 083H □	8" 083H □
10" 103H □	10" 103H □
12" 123H □	12" 123H □

Refer to drawing NS415 and NS416 for jamb and header identification.

Blanket Insulation Detail at Eave

Note: Fold insulation back onto itself for compression between Eave Strut and roof panel, it must not be exposed to the weather.

Ridge Tie Installation

Roof slope	8" Purlin	10" Purlin	12" Purlin
1/2	RT-08 A	RT-10 A	RT-12 A
1	RT-08 B	RT-10 B	RT-12 B
2	RT-08 C	RT-10 C	RT-12 C
3	RT-08 D	RT-10 D	RT-12 D
4	RT-08 E	RT-10 E	RT-12 E

Knock-In Bridging Attachment
Low Side Eave Strut

All fasteners shown are #18 1/4" - 14 x 1-1/4" Self Drill (3) Required

Knock-In Bridging Attachment
At Interior Purlin

Insert KBA Tab Into Purlin Web Slot And Bend Over Flat Against the Purlin Web.

NS0017

NOTES:

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NS0094B

NS0059B

NS0410

NS0412

Standard Details

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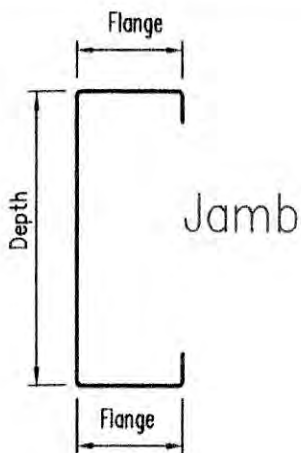
Refer to the Technical Installation Information Manual for the installation of the Roof Panel.

Cold Formed Steel Components Yield Strength = 57.0 K.S.I. (Typ.)

Section Name	Dimensions			Member Properties						Shape
	D	B1	B2	Lip	Ga.	Area	Ix	Iy	Sx	
08226	8	2 1/2	2 1/2	.911	16	0.822	7.759	1.081	1.747	Zees
08224	8	2 1/2	2 1/2	.930	14	0.976	9.177	1.289	2.137	
08223	8	2 1/2	2 1/2	.956	13	1.185	11.095	1.577	2.724	
08222	8	2 1/2	2 1/2	.990	12	1.463	13.624	1.967	3.347	
10224	10	2 1/2	2 1/2	.930	14	1.116	15.642	1.290	2.858	Zees
10223	10	2 1/2	2 1/2	.956	13	1.355	18.933	1.578	3.727	
10222	10	2 1/2	2 1/2	.990	12	1.673	23.285	1.968	4.586	
10324	10	3 1/2	3 1/2	.930	14	1.256	19.084	3.077	2.907	
10323	10	3 1/2	3 1/2	.956	13	1.525	23.112	3.759	3.933	Zees
10322	10	3 1/2	3 1/2	.990	12	1.883	28.427	4.681	4.932	
12324	12	3 1/2	3 1/2	.930	14	1.396	29.321	3.078	3.470	
12323	12	3 1/2	3 1/2	.956	13	1.695	35.515	3.760	4.733	
12322	12	3 1/2	3 1/2	.990	12	2.093	43.722	4.682	6.374	Zees
082C6	8	2 1/2	2 1/2	.773	16	0.822	7.791	0.675	1.668	
082C4	8	2 1/2	2 1/2	.800	14	0.976	9.210	0.800	2.049	
082C2	8	2 1/2	2 1/2	.885	12	1.463	13.649	1.196	3.412	
083C6	8	3 1/2	3 1/2	.773	16	0.940	9.652	1.523	1.712	
083C4	8	3 1/2	3 1/2	.800	14	1.116	11.411	1.808	2.251	Zees
083C2	8	3 1/2	3 1/2	.885	12	1.673	16.321	2.712	3.574	
103C2	10	3 1/2	3 1/2	.885	12	1.883	28.436	2.912	4.861	
123C2	12	3 1/2	3 1/2	.885	12	2.093	43.758	3.071	6.293	

Section Name	Dimensions			Member Properties						Shape
	D	B1	B2	Lip	Ga.	Area	Ix	Iy	Sx	
083EL6	8	3 1/2	5	1.088	16	1.058	10.936	2.902	1.890	Eave Struts
083EL4	8	3 1/2	5	1.114	14	1.256	12.924	3.440	2.499	
083EL2	8	3 1/2	5	1.200	12	1.883	19.143	5.142	4.001	
103EL4	10	3 1/2	5	1.114	14	1.396	21.670	3.697	3.161	
103EL2	10	3 1/2	5	1.200	12	2.093	32.195	5.537	5.458	Eave Struts
123EL4	12	3 1/2	5	1.114	14	1.536	33.201	3.907	3.815	
123EL2	12	3 1/2	5	1.200	12	2.303	49.422	5.855	7.067	

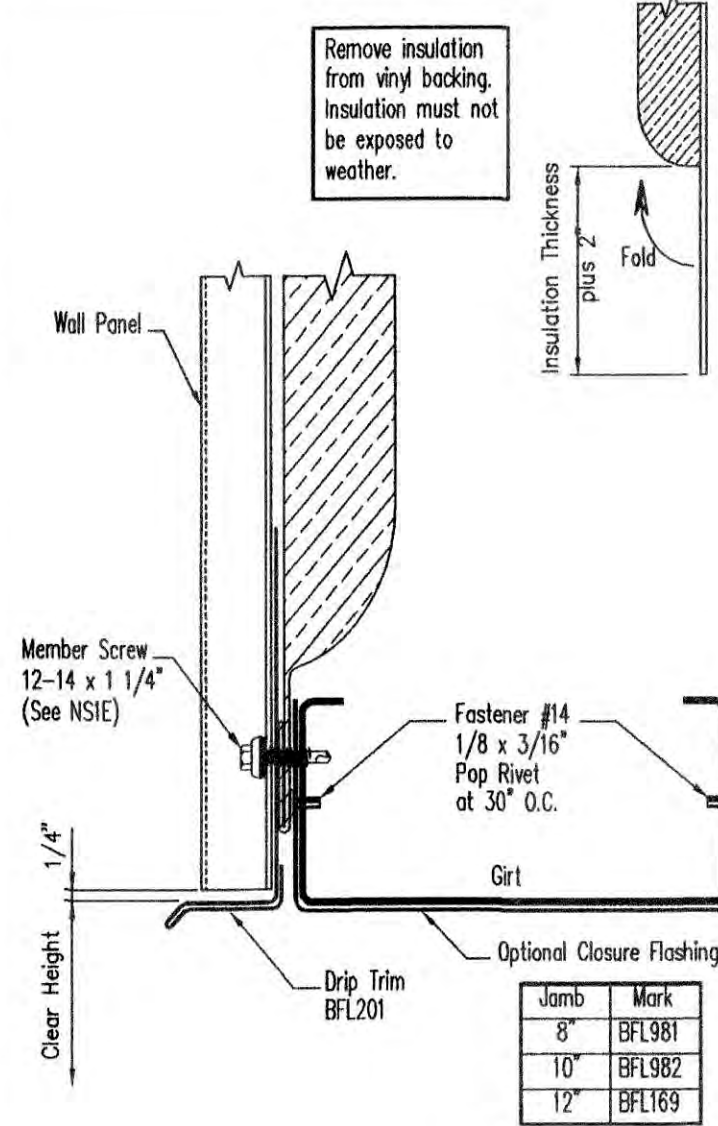
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083EL4	8	3 1/2	5	1.114	14	1.256	12.924	3.440	2.499	
083EL2	8	3 1/2	5	1.200	12	1.883	19.143	5.142	4.001	
103EL4	10	3 1/2	5	1.114	14	1.396	21.670	3.697	3.161	
103EL2	10	3 1/2	5	1.200	12	2.093	32.195	5.537	5.458	Eave Struts
123EL4	12	3 1/2	5	1.114	14	1.536	33.201	3.907	3.815	
123EL2	12	3 1/2	5	1.200	12	2.303	49.422	5.855	7.067	



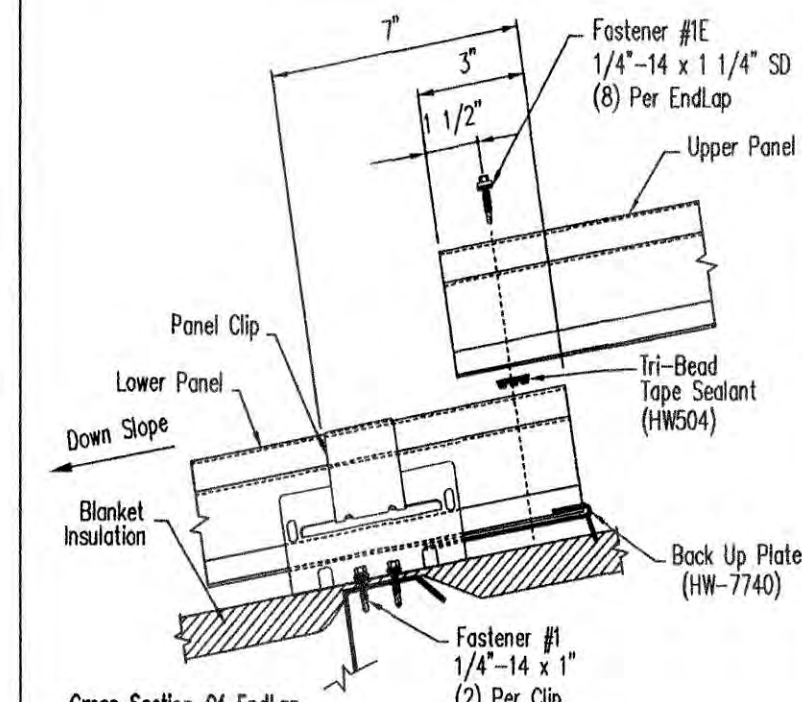
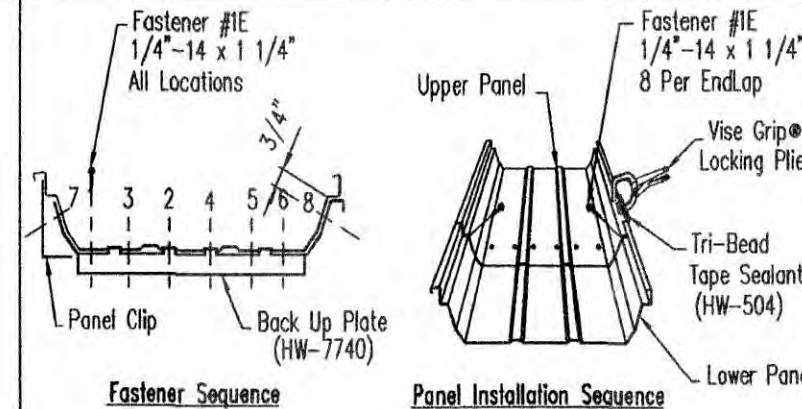
Depth Flange Profile Gage Base Top Counter Slope View
08 3 J 4 F L - 1 B R

- a. Depth Code
08 = 8"
10 = 10"
12 = 12"
- b. Flange Code
3 = 3 1/2"
- c. Profile Code
J = Jamb
- d. Gage Code
2 = 12 ga.
3 = 13 ga.
4 = 14 ga.
6 = 16 ga.
- e. Base Code
F = Floor
Z = Zee
C = Cee
- f. Top Code
L = Low Eave
H = High Eave
Z = Zee
C = Cee
R = Rafter
E = End Rafter
- g. Counter
A numerical part number from 1 to 999
- h. Slope Code
A = 1/2:12
B = 1:12
C = 2:12
D = 4:12
- i. View Code
R = As Shown
L = Opposite as Shown

Standard Jamb Identification



Open Wall with Cee Girl



(UD/DL) Standing Seam Panel Lap Endlap

Mid Slope Fixed Condition - UD/DL End Lap - Low System

NS0266

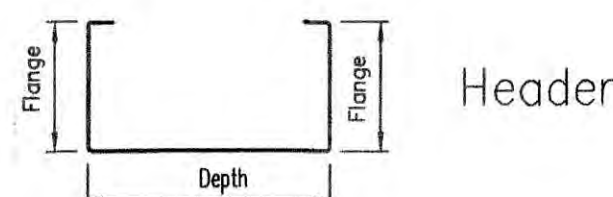
DEPTH	FLANGE	PROFILE	GAGE	LEFT	RIGHT	COUNTER
8	3	C	4	G	G	1
Depth Code						
8 = 8"						
10 = 10"						
12 = 12"						
Flange Code						
3 = 3 1/2"						
Zees						
DEPTH	FLANGE	PROFILE	GAGE	LEFT	RIGHT	COUNTER
8	3	Z	4	G	G	1
Depth Code						
8 = 8"						
10 = 10"						
12 = 12"						
Flange Code						
2 = 2 1/2"						
3 = 3 1/2"						
Profile Code						
Z = Zee						

DEPTH	FLANGE	PROFILE	GAGE	LEFT	RIGHT	COUNTER
8	3	EL	4	H	H	1
Depth Code						
8 = 8"						
10 = 10"						
12 = 12"						
Flange Code						
3 = 3 3/8"						
5 = 4 1/2" or 5"						
Profile Code						
EL = Eave Low						
EH = Eave High						

DEPTH	FLANGE	PROFILE	GAGE	LEFT	RIGHT	COUNTER	SLOPE
8	3	EL	4	H	H	1	A
Depth Code							
8 = 8"							
10 = 10"							
12 = 12"							
Flange Code							
3 = 3 3/8"							
5 = 4 1/2" or 5"							
Profile Code							
EL = Eave Low							
EH = Eave High							

Cold Form Identification

NS0067

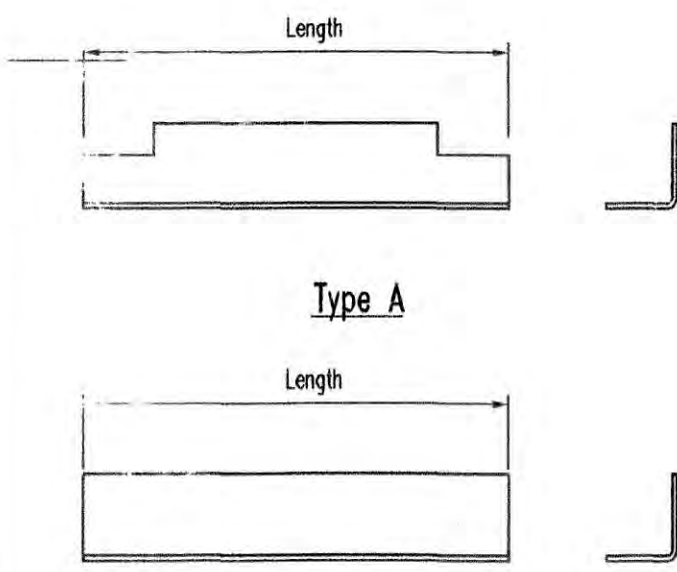


Depth Flange Profile Gage Counter
08 3 H 4 - 1

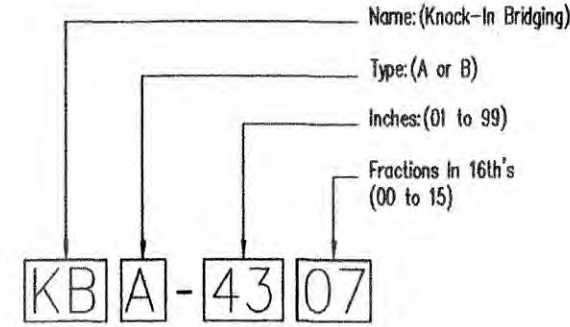
- a. Depth Code
08 = 8"
10 = 10"
12 = 12"
- b. Flange Code
3 = 3 1/2"
- c. Profile Code
H = Header
- d. Gage Code
2 = 12 ga.
3 = 13 ga.
4 = 14 ga.
6 = 16 ga.
- g. Counter
A numerical part number from 1 to 999

Standard Header/Sill Identification

NS0415

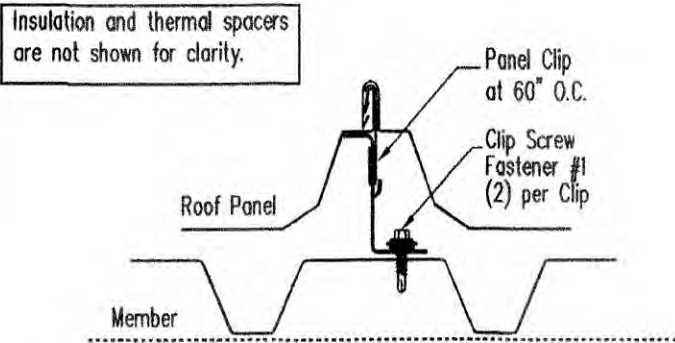


Angle 1 x 1 x 14ga.

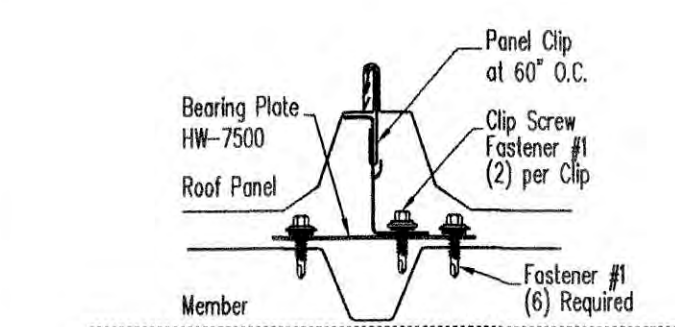


Knock-In-Bridging Identification

NS0818



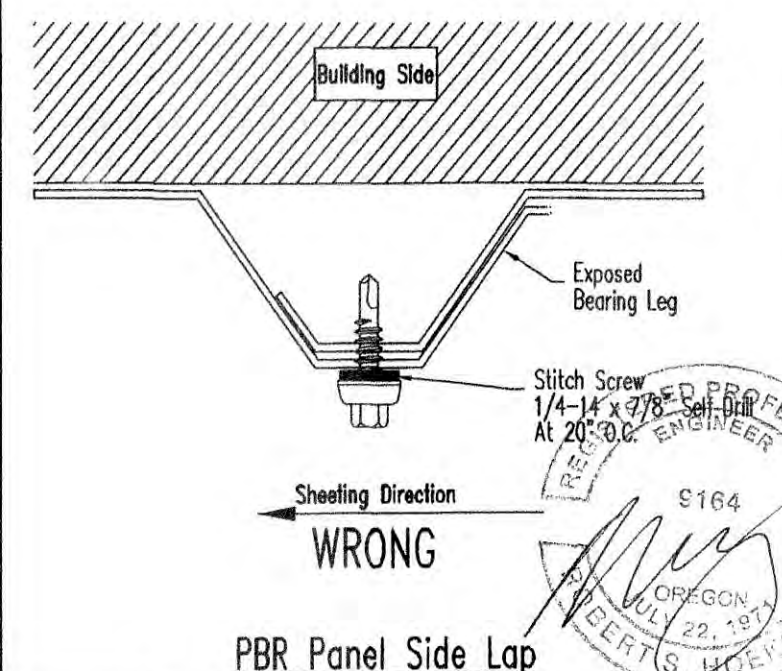
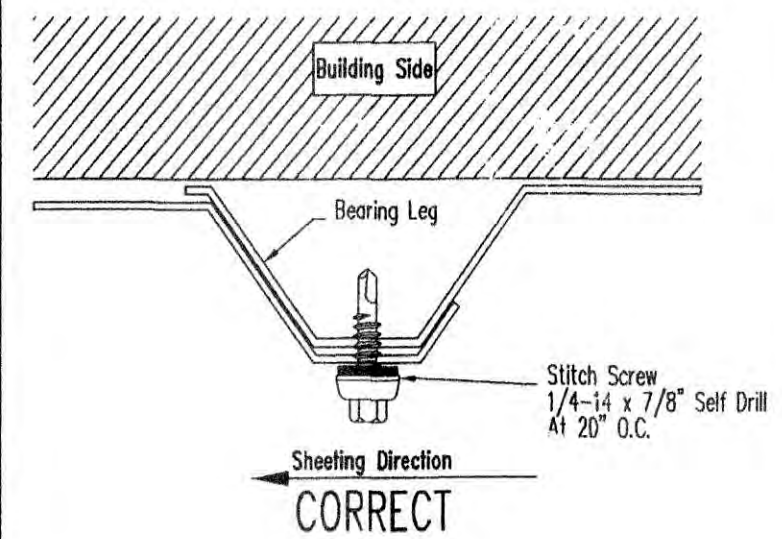
BW36 Panel Flat



BW36 Panel Rib

Panel Clip Installation
UD/DL Roof-BW36 Panel-Blanket Insulation

NS0187



PBR Panel Side Lap

NS0067A

NS0416

NS0059J

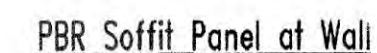
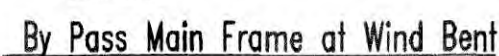
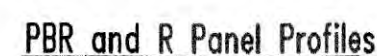
NS0488

NS0016A

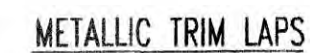
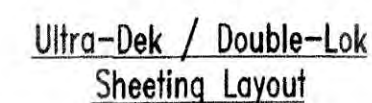
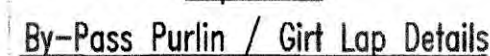
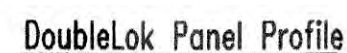
NOTE:
1. These drawings are intended to depict general installation of item(s) described above. Some item(s) may have been omitted for clarity of presentation. Consult your erection manual or additional S-Sheets for further guidelines and/or clarifications.
ATTENTION INSTALLER: In the event that a discrepancy or error arises with materials shipped for this project or these construction drawings, the installer must notify the manufacturer prior to correcting. If manufacturer is not notified, manufacturer will

REVISIONS				
NO.	DATE	DESCRIPTION	BY	CHK'D
A		FOR CONSTRUCTION PERMIT		

Standard Details									
Customer: HEDGES CREEK MANAGEMENT # 1 LLC									
End User: HEDGES CREEK MANAGEMENT # 1 LLC									
Location: ACAPPASE, OR									
Det. By	Chk By	Date	Scale	Building ID	Job Number	Sheet Number	Issue		
			N.T.S.	A	0805-236857	NS-6	A		



NS0117B



NS0080

Customer: HEDGES CREEK MANAGEMENT # 1 LLC						
End User: HEDGES CREEK MANAGEMENT # 1 LLC						
Location: ACAPASE, OR						
Det. By	Chk By	Date	Scale	Building ID	Job Number	Sheet Number
			N.T.S.	A	0805-236857	NS-7

Standard Details

